APPENDIX C.9 CASE 10-T-0139 SITE PLANS AND CONSTRUCTION DRAWINGS SITE PLAN ASTORIA HVDC CONVERTER STATION SEGMENT 22



ASTORIA HVDC CONVERTER STATION

SITE STRUCTURAL PACKAGE

SCOPE OF WORK

THE BUILDING STRUCTURAL SCOPE OF WORK INCLUDES THE DESIGN OF STRUCTURAL SYSTEMS INCLUDING ACCESS REQUIREMENTS FOR THE ASSOCIATED LOCATIONS INDICATED BELOW:

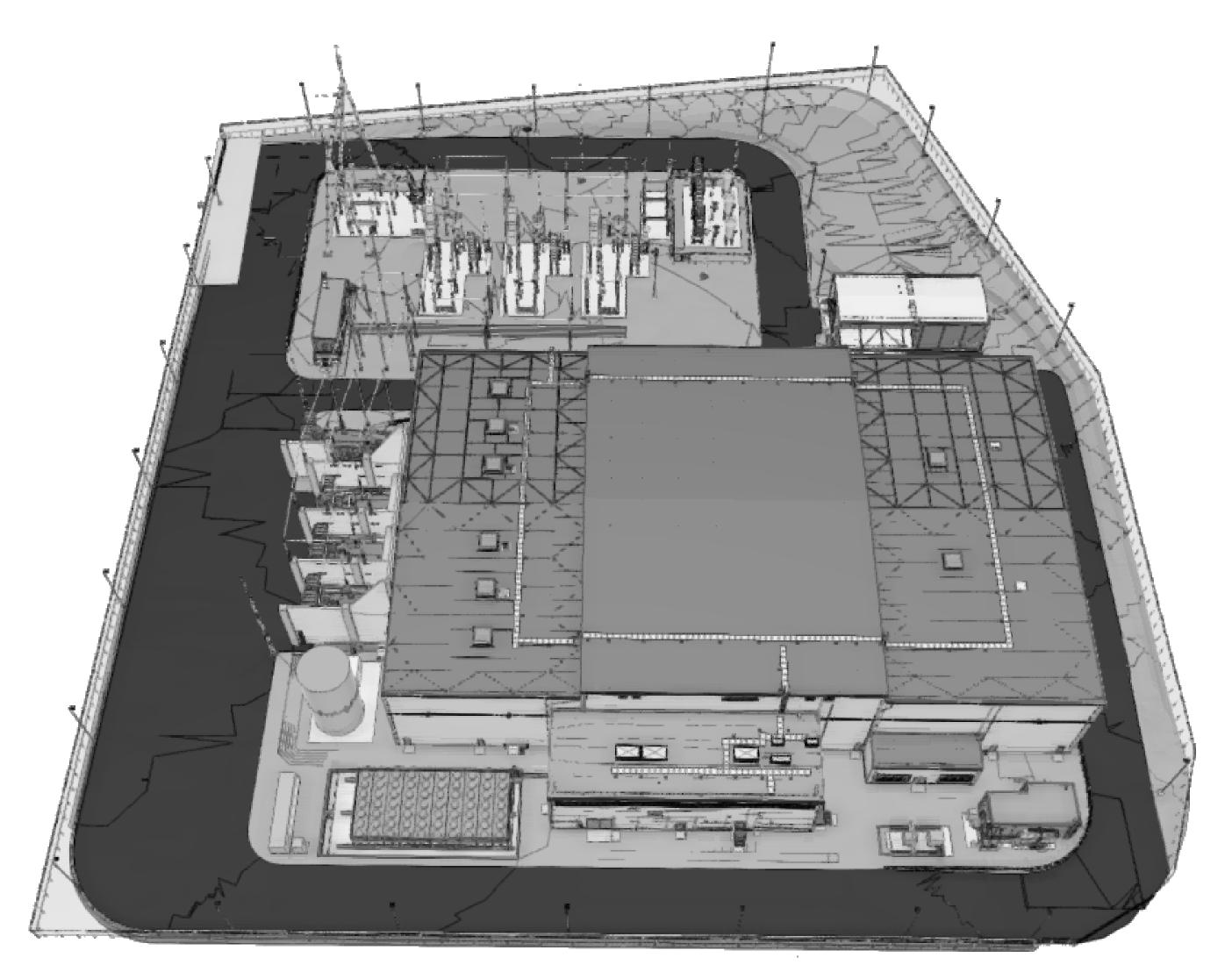
- AC YARD EQUIPMENT AND GANTRY
 TRANSFORMERS AREA AND TRANSFORMER BLAST WALLS
- 3. SPARE TRANSFORMER AREA
- 4. FIRE WATER TANK AND FIRE PUMP ENCLOSURE
- 4. FIRE WATER TANK AND FIRE PUN 5. AUXILARY TRANSFORMERS
- 6. SITE WIDE ELECTRICAL SUPPORTS

FLOOD ZONE DESIGN CERTIFICATION:

THE EXISTING PROPERTY IS IN THE SPECIAL FLOOD HAZARD AREA (SFHA), ZONE AE PER EFFECTIVE 2015 FLOOD INSURANCE RATE MAP(FIRM). THIS IS TO CONFIRM THAT THE PROPOSED INSTALLATION IS IN COMPLIANCE WITH THE REQUIREMENTS SET FORTH IN APPENDIX Q OF THE NYC BUILDING CODE.

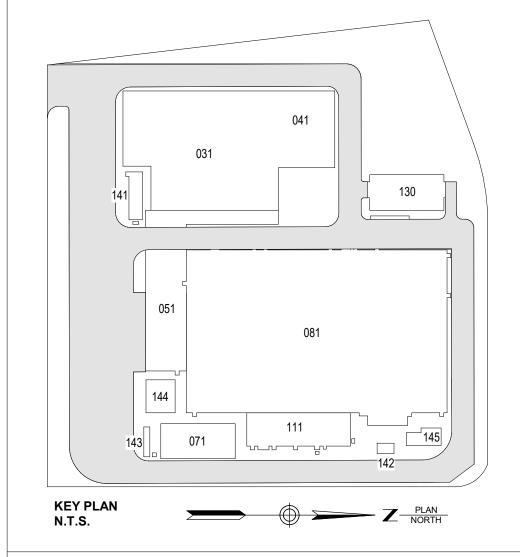
NYC ENERGY CODE COMPLIANCE:

STATEMENT: TO THE BEST OF MY KNOWLEDGE, BELIEF AND PROFESSIONAL JUDGEMENT, THESE PLANS AND SPECIFICATIONS ARE IN COMPLIANCE WITH THE NEW YORK CITY ENERGY CONSERVATION CODE. PROPOSED WORK MEETS THE GUIDELINES AND INSTRUCTIONS OUTLINED IN THE 2020 NYC ECC CHAPTER 4.





ISSUED FOR PERMIT





370 7th Avenue SUITE 1604 New York, NY 1000



25 Mohawk Avenue Sparta, NJ 07871

CONFIDENTIA

THESE DRAWINGS ARE CONFIDENTIAL IN NATURE. ANY MISUSE OR UNAUTHORIZED DISTRIBUTION OF THE DRAWINGS CONTAINED HEREIN WILL BE A VIOLATION OF THIS CONFIDENTIALITY REQUIREMENT AND SUBJECT THE VIOLATOR TO LIABILITY. REVIEW OF THESE MATERIALS BY RECEIPT SHALL CONSTITUTE ACCEPTANCE OF THESE TERMS AND THE TERMS OF ANY UNDERLYING CONFIDENTIALITY AGREEMENT WE MAY HAVE EXCLUDED IN OBTAINING THIS INFORMATION FROM A THIRD PARTY. IF THE RECIPIENT IS NOT IN AGREEMENT WITH THE OBLIGATION OF CONFIDENTIALITY THEN THE DRAWINGS CHALL BE RETURNED TO THE ORIGINATOR

В	FINAL SUBMISSION	VSP	EK	12/12/2022
Α	INTERIM SUBMISSION	VSP	EK	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



Hitachi Energy901 Main Campus DriveRaleigh, North Carolina 27606

PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

COVER SHEET

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	V. PATEL
CHECKED BY	E. KIDANE
DRAWING NO	
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JD FILE INU

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ia/CHA-KIE-000-XX-M2-S-001.rvt

GN GENERAL REQUIREMENTS GN-1. THE DESIGN DRAWINGS ARE NOT TO BE CONSIDERED ALL INCLUSIVE, AND IT IS THE FIELD PERSONNEL'S RESPONSIBILITY TO VERIFY ALL EXISTING CONDITIONS AND VERIFY ALL DIMENSIONS BEFORE PROCEEDING WITH FOUNDATION INSTALLATION OR ANY OTHER CONSTRUCTION. ANY DISCREPANCIES FOUND BETWEEN THE DESIGN DRAWINGS AND THE ACTUAL FIELD CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD FOR REEVALUATION BEFORE PROCEEDING WITH WORK. GN-2. IN CASE OF CONFLICT BETWEEN DESIGN DRAWINGS AND SPECIFICATIONS, THE ENGINEER OF RECORD SHALL BE NOTIFIED TO OBTAIN CLARIFICATION PRIOR TO PROCEEDING WITH WORK. GN-3. ONLY USE DIMENSIONS INDICATED ON THE DESIGN DRAWINGS. DO NOT SCALE DES DRAWINGS. GN-4. STRUCTURES HAVE BEEN DESIGNED TO BE STABLE IN THEIR FINAL STATE. CONTRACTOR TO ENGAGE A QUALIFIED ENGINEER FOR ALL TEMPORARY CONDIT ERECTION AIDS, LIFTING DEVICES, ETC. ARE NOT SHOWN AND ARE THE RESPONSI OF THE ERECTOR'S ENGINEER OR AS APPROVED BY THE ENGINEER OF RECORD. CS CODES AND SPECIFICATIONS CS-1. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE FOLLOWING CODES a. NFPA 850: RECOMMENDED PRACTICE FOR FIRE PROTECTION FOR ELECTRIC GENERATING PLANTS AND HIGH VOLTAGE DIRECT CURRENT CONVERTER STAT b. NEW YORK CITY BUILDING CODE, 2022 c. INTERNATIONAL BUILDING CODE, 2015 AS MODIFIED BY NYCBC d. MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, ASCE/SEI e. ASCE 113-2008 SUBSTATION STRUCTURE DESIGN GUIDE. f. ASCE 48-19 DESIGN OF STEEL TRANSMISSION POLE STRUCTURES. g. BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, ACI 318-2014 A MODIFIED BY NYBC1908. h. SPECIFICATIONS FOR STRUCTURAL CONCRETE, ACI 301-2010 i. MANUAL OF STANDARD PRACTICE, CRSI MSP-1 2009 BUILDING CODE REQUIREMENTS AND SPECIFICATION FOR MASONRY STRUCTU ASCE 5-13 OR TMS 402/602-16 k. STEEL CONSTRUCTION MANUAL - 15TH EDITION, AISC 325-2015 I. SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, AISC 360-2016 m. STRUCTURAL WELDING CODE – STEEL, AWS D1.1/D1.1M:2015 n STRUCTURAL WELDING CODE – REINFORCING STEEL, STEEL REINFORCING BAI AWS D1.4/D1.4M:2018 o. STANDARD FOR NON-COMPOSITE STEEL FLOOR DECK, ANSI/SDI NC1.0- 2017 p. STANDARD FOR STEEL ROOF DECK, ANSI/SDI RD1.0- 2017 q. STANDARD FOR COMPOSITE STEEL FLOOR DECK - SLABS, SDI C- 2017 r. STANDARD FOR QUALITY CONTROL AND QUALITY ASSURANCE FOR INSTALLATI OF STEEL DECK, SDI QA/QC- 2017 s. OCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) STANDARDS, DEPARTMENT OF LABOR, PART 1910 AND PART 1926 CS-2. ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE FOLLOWING SPECIFICATIONS: a. SECTION 03 10 00, CONCRETE FORMING AND ACCESSORIES b. SECTION 03 20 00, CONCRETE REINFORCING c. SECTION 03 30 00, CAST-IN-PLACE CONCRETE d. SECTION 03 41 00, PRECAST STRUCTURAL CONCRETE e. SECTION 03 60 00, GROUTING f. SECTION 05 05 13, GALVANIZING g. SECTION 05 05 23.01, WELDING h. SECTION 05 05 23.02, MISC METAL FASTENINGS i. SECTION 05 12 00, STRUCTURAL STEEL FRAMING j. SECTION 05 31 13, STEEL FLOOR DECKING k. SECTION 05 31 23, STEEL ROOF DECKING I. SECTION 05 40 00, COLD FORMED METAL FRAMING m. SECTION 05 50 00, METAL FABRICATION n. SECTION 05 51 00, METAL STAIRS o. SECTION 05 52 13, PIPE AND TUBE RAILINGS p. SECTION 05 53 00, METAL GRATINGS q. SECTION 31 09 16, DRIVEN PILE LOAD TESTING r. SECTION 31 20 00, EARTH MOVING s. SECTION 31 23 19, DEWATERING t. SECTION 31 62 00, DRIVEN PILES u. SECTION 31 62 16, STEEL PILES (INCLUDING SHEET PILES FOR EXCAVATION <u>DL DESIGN LOADS</u> DL-1. REFER TO LOAD DIAGRAMS FOR SPECIFIC CONDITIONS DL-2. RISK CATEGORY. DL-3. MINIMUM LIVE LOADS: a. CATWALKS.. .250 PSF b. CONTROL ROOMS.. ..75 PSF + ACTUAL EQUIPMENT WEIGHT c. ELECTRICAL EQUIPMENT ROOMS... d. FIRE PROTECTION SPRINKLER PIPING SUPPORT ...5x WATER WT + 250 LB

e. ISOLATED PLATFORM FOR SERVICING EQUIPMENT

f. PLATFORMS & WALKWAYS

g. ROOF LIVE LOAD.

h. SLABS-ON-GRADE..

i. STAIRS AND RAMPS..

j. STORAGE AREA..

.150 PSF

..100 PSF

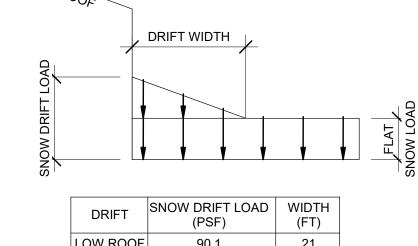
.250 PSF

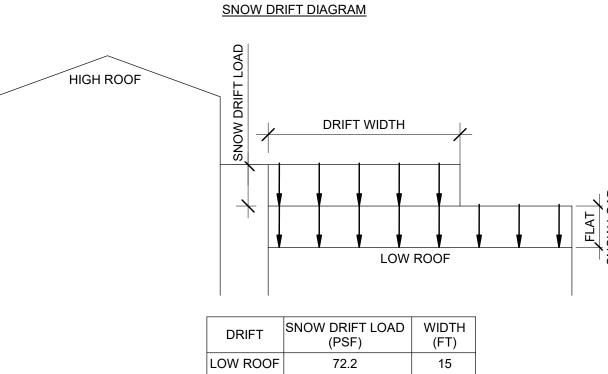
..100 PSF

..250 PSF

		b. BASIC WIND SPEED (V _{ULT})
		c. NOMINAL WIND SPEED (V _{ASD})
ESIGN		d. EXPOSURE CATEGORY
	DL-5.	SEISMIC LOADS:
TIONS. SIBILITY		a. IMPORTANCE FACTOR (I _e)
		b. SITE CLASS
		c. MAPPED SPECTRAL RESPONSE ACCELERATIONS:
2 4115		i. 0.2 SECOND SHORT PERIOD (S _S)
S AND		ii. 1.0 SECOND PERIOD (S ₁)
		d. DESIGN SPECTRAL RESPONSE ACCELERATIONS:
ATIONS		i. 0.2 SECOND SHORT PERIOD (S _{DS})
		ii. 1.0 SECOND PERIOD (S _{D1})
		e. SEISMIC DESIGN CATEGORY
I		f. SEISMIC RESPONSE COEFICIENT
		g. RESPONSE MODIFICATION FACTOR
		h. OVER STRENGTH FACTOR
AS		
		j. BASIC SEISMIC FORCE RESISTING SYSTEMSTEEL SYSTEM NOT SPECIFI DETAILED FOR SEISMIC RES
	DL-6.	SNOW LOADS:
URES,		a. IMPORTANCE FACTOR (I _s)
,		b. GROUND SNOW LOAD (pg)
		c. EXPOSURE FACTOR (C _e):
		d. THERMAL FACTOR (Ct):
		e. FLAT ROOF SNOW LOAD (pf)2
ARS,	DL-7.	SERVICEABILITY
		a. ROOF MEMBERS - VERTICAL DEFLECTION:
		i. LIVE
		ii. DEAD + LIVE
TION		b. FLOOR MEMBERS - VERTICAL DEFLECTION:
HON		i. LIVE
		ii. DEAD
		c. GIRTS:
		i. VERTICAL DEFLECTION
		ii. LATERAL DEFLECTION
		d. LATERAL DRIFT DUE TO 10-YR MRI WIND LOADS:
		i. BUILDINGS
		ii. PIPE RACK AND SIMILAR OPEN STRUCTURES
	DL-8.	REFER TO VENDOR DOCUMENTATION FOR SPECIFIC EQUIPMENT FOUNDATION LO
	22 0.	SERVICEABILITY INFORMATION.
	SNOW	LOAD DIAGRAMS
		HIGH ROOF
		DRIFT WIDTH
		OAD
		NRIFT C
		OW DR
		SNOW DRIFT LOAD
		DRIFT SNOW DRIFT LOAD WIDTH (PSF) (FT)
		LOW ROOF 90.1 21
		SNOW DRIFT DIAGRAM
		0
		HIGH ROOF OF
	/	

CS-3. SURCHARGE ADJACENT TO STRUCTURES: a. AASHTO DESIGN TRUCK LOADING... b. SIDEWALK, VEHICULAR DRIVEWAYS SUBJECTED AND YARD SUBJECTED TO TRUCKING... DL-4. WIND LOADS: a. IMPORTANCE FACTOR (I_w). b. BASIC WIND SPEED (VULT). ...132 MPH ..102 MPH0.061CS=0.10 OCEDURE FICALLY SISTANCE25 PSF .24.15 PSFL/240L/180H/400H/200 LOAD AND





SLIDING SNOW DIAGRAM

CM CONCRETE MATERIALS

- CM-1. CONCRETE MIX DESIGN, PLACEMENT, AND CURING SHALL BE IN ACCORDANCE WITH ACI
- CM-2. USE A MINIM 28-DAY CONCRETE COMPRESSIVE STRENGTH OF 5,000 PSI UNLESS NOTED
- CM-3. ALL EXTERIOR FOUNDATIONS SHALL BE BROOM FINISHED, UNLESS NOTED OTHERWISE. ALL INTERIOR SLABS SHALL BE SMOOTH TROWEL FINISHED UNLESS NOTED
- CM-4. ALL MASS CONCRETE WILL BE INDICATED ON THE INDIVIDUAL FOUNDATION AND CONCRETE DESIGN DRAWINGS. PLACEMENTS OF MASS CONCRETE SHALL BE INSTALLED IN ACCORDANCE WITH THERMAL CONTROL PLANS AND BE APPROVED BY THE ENGINEER OF RECORD.
- CM-5. CONCRETE SHALL BE CURED ACCORDING TO ACI 308.1. CONCRETE SHALL BE PROTECTED FROM LOSS OF MOISTURE FOR NOT LESS THAN SEVEN DAYS AFTER PLACEMENT AND WITH NECESSARY PROTECTION FOR COLD OR HOT WEATHER
- CM-6. THE USE OF CALCIUM CHLORIDE AND OTHER CHLORIDE-CONTAINING AGENTS IS PROHIBITED. THE USE OF RECYCLED CONCRETE IS PROHIBITED. PLACEMENT WITHIN/CONTACT BETWEEN ALUMINUM ITEMS (INCLUDING ALUMINUM CONDUIT) AND
- CONCRETE IS PROHIBITED. CM-7. ALL PERMANENTLY EXPOSED CONCRETE EDGES SHALL HAVE A 3/4" CHAMFER UNLESS
- CM-8. CONSULT MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENINGS AND EMBEDDED ITEMS SUCH AS FLOOR DRAIN SYSTEMS, CONDUIT, ETC.
- CM-9. OBSERVABLE CRACKS SHALL BE REPORTED TO THE ENGINEER OF RECORD TO DETERMINE CAUSE AND APPROPRIATE REPAIR PROCEDURE.
- CM-10. PERFORM CONCRETE TESTING IN ACCORDANCE WITH SPECIFICATIONS.

RE CONCRETE REINFORCEMENT

- RE-1. REINFORCING BAR STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM A615 GRADE 60 AND GRADE 80 DEFORMED BARS UNLESS NOTED OTHERWISE. WELDED WIRE REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A1064. MILL TEST CERTIFICATES SHALL BE PROVIDED IN ACCORDANCE WITH SPECIFICATIONS.
- RE-2. FABRICATION AND PLACEMENT OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH CRSI MSP-1 AND ACI 301.
- RE-3. CHAIRS, SPACERS, STANDEES, AND CARRIER BARS SHALL BE SIZED, SPACED, AND PLACED TO PROVIDE THE REQUIRED SPACING, ALIGNMENT, AND CLEARANCES OF REINFORCING. CARRIER BARS SHALL NOT BE USED AS PRIMARY REINFORCING BARS.
- RE-4. REINFORCING BAR LAP SPLICES NOT OTHERWISE INDICATED SHALL BE ACI CLASS B. WELDED WIRE REINFORCEMENT SHALL BE LAPPED ONE PANEL PLUS TWO INCHES
- RE-5. WHERE A 90-DEGREE, 135-DEGREE, OR 180-DEGREE HOOK IS GRAPHICALLY INDICATED, PROVIDE CORRESPONDING ACI 318-14 STANDARD HOOKS UNLESS NOTED OTHERWISE.
- RE-6. DOWELS SHALL MATCH SIZE AND SPACING OF MAIN REINFORCEMENT UNLESS NOTED
- RE-7. REINFORCING STEEL SHALL BE SECURELY TIED IN PLACE.
- RE-8. ALL BENDING OF REINFORCEMENT SHALL BE DONE COLD.
- RE-9. PROVIDE MECHANICAL SPLICES FOR BARS LARGER THAN #11 OR WHERE INDICATED. ALL MECHANICAL SPLICES SHALL BE APPROVED BY THE ENGINEER OF RECORD.
- RE-10. WELDING OF REINFORCING IS NOT PERMITTED UNLESS APPROVED BY THE ENGINEER
- RE-11. PROVIDE MIN CONCRETE COVER OVER REINFORCING STEEL AS FOLLOWS UNLESS NOTED OTHERWISE:

	CONCRETE EXPOSURE	MEMBER	REINFORCEMENT	SPECIFIED COVER, IN.
	CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND	ALL	ALL	3
	EXPOSED TO WEATHER		#6 THROUGH #18 BARS	2
O	OR IN CONTACT WITH GROUND	ALL	#5 BAR, W31 OR D31 WIRE AND SMALLER	1-1/2
		SLABS, JOISTS, AND	#14 AND #18 BARS	1-1/2
	NOT EXPOSED TO	WALLS	#11 BAR AND SMALLER	3/4
	WEATHER OR IN CONTACT WITH GROUND	WEATHER OR IN PEAMS COLLINNS		1-1/2

RE-12. PROVIDE SPLICES, DEVELOPMENT, AND STANDARD HOOKS AS FOLLOWS UNLESS

REINFORCING STEEL SPLICE CHART FOR F'c = [5000 PSI]								
	LEN	SPLICE LENGTH (CLASS B)		OPEMEN GNTH	DEVELOPEMEN T LENGTH FOR STANDARD	LENGTH OF STANDARD		
BAR SIZE	TOP BARS	OTHER BARS	TOP BARS	P OTHER HOOKS		HOOKS		
#3	22"	22" 17" 17" 13" 29" 22" 22" 17"		6"	7"			
#4	29"			17"	9"	9"		
#5	36"	27"	28"	21"	11"	11"		
#6	43"	33"	33"	25"	13"	14"		
#7	62"	48"	48"	37"	15"	16"		
#8	72"	55"	55"	42"	17"	18"		
#9	81"	62"	62"	48"	19"	23"		
#10	91"	70"	70"	54"	22"	25"		
#11	101"	78"	78"	60"	24"	28"		

CJ CONCRETE CONSTRUCTION JOINTS

THE ENGINEER OF RECORD.

- CJ-1. SEE DESIGN DRAWINGS FOR ALL CONSTRUCTION JOINT, CRACK CONTROL JOINT, EXPANSION JOINT, AND ISOLATION JOINT LOCATIONS.
- CJ-2. NO HORIZONTAL CONSTRUCTION JOINTS SHALL BE PERMITTED IN BEAMS, WALLS, OR SLABS UNLESS SPECIFICALLY SHOWN ON THE DESIGN DRAWINGS OR APPROVED BY
- CJ-3. PROVIDE CONTINUOUS WATERSTOPS AT ALL CONSTRUCTION JOINTS EXPOSED TO SOIL OR WATER ON THE DESIGN DRAWINGS UNLESS NOTED OTHERWISE. INSTALL PER
- SPECIFICATIONS AND MANUFACTURER'S RECOMMENDATIONS. CJ-4. WATERSTOPS SHALL BE FOUR-INCH RIBBED CENTERBULB-TYPE POLYVINYL CHLORIDE

PER SPECIFICATIONS UNLESS NOTED OTHERWISE.

- CJ-5. FOR ALL CONSTRUCTION JOINTS ROUGHEN EXPOSED CONCRETE SURFACE TO AN AMPLITUDE OF APPROXIMATELY 1/4" UNLESS NOTED OTHERWISE. CLEAN THE EXPOSED CONCRETE SURFACE OF ALL LOOSE MATERIAL AND LAITANCE.
- CJ-6. SAWCUT JOINTS SHALL BE CUT AS SOON AS THE CONCRETE HAS HARDENED SUFFICIENTLY TO PREVENT AGGREGATE BEING DISLODGED BY SAW; GENERALLY, WITHIN FOUR HOURS AFTER PLACING IN HOT WEATHER AND NOT MORE THAN 12 HOURS IN COLD WEATHER.

SP STRUCTURAL PRECAST CONCRETE

- SP-1. DO NOT USE POWER-DRIVEN ANCHORS OR ANCHORS WHICH REQUIRE DRILLING AT PRESTRESSED UNITS. SUBMIT PROPOSED ANCHOR PROCEDURES FOR PRECAST UNITS TO THE ENGINEER OF RECORD AND PRECAST SUPPLIER FOR REVIEW.
- SP-2. ALL PRECAST DESIGN, DETAILING, AND CONNECTIONS SHALL BE IN ACCORDANCE WITH RECOMMENDATIONS OF PCI AND SPECIFICATIONS.

GT GROUT

- GT-1. GROUT SHALL BE NON-SHRINK, NON-METALLIC, NON-GASEOUS, PREMIX TYPE UNLESS NOTED OTHERWISE. COMPRESSIVE STRENGTH OF CEMENTITIOUS GROUT SHALL BE MINIMUM 5000 PSI AT 28 DAYS. COMPRESSIVE STRENGTH OF EPOXY GROUT SHALL BE MINIMUM 12,000 PSI AT SEVEN DAYS. CURING SHALL FOLLOW MANUFACTURER'S RECOMMENDATIONS.
- GT-2. THE ORDER OF PRECEDENCE FOR GROUTING OF MACHINERY AND EQUIPMENT SHALL BE AS FOLLOWS: EQUIPMENT MANUFACTURER'S INSTALLATION INSTRUCTIONS, DESIGN DRAWINGS, AND THEN SPECIFICATIONS. IN THE EVENT OF CONFLICT BETWEEN THESE DOCUMENTS, NOTIFY THE ENGINEER OF RECORD PRIOR TO PROCEEDING WITH WORK.

ISSUED FOR PERMIT



370 7th Avenue **SUITE 1604** New York, NY 10001



25 Mohawk Avenue **Sparta, NJ 07871**

CONFIDENTIAL

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@Hitachi Energy 901 Main Campus Drive Raleigh, North Carolina 27606

PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

STRUCTURAL GENERAL **NOTES**

> 12/12/2022 PROJECT NO 105121 DRAWING BY D. FLYNN CHECKED BY W. ABBASSI

DRAWING NO

Autodesk Docs://CHPE Astoria/CHA-KIE-000-XX-M2-S-001.rvt

PA POST INSTALLED ANCHORS

- PA-1. EXPANSION ANCHOR BOLTS SHALL BE HILTI KWIK BOLT TZ (ICC ESR-1917) OR APPROVED EQUAL IN ACCORDANCE WITH ICC-ES AC193 AND SHALL BE STAINLESS STEEL SS 304 STAINLESS FOR EXTERIOR USE UNLESS NOTED OTHERWISE.
- PA-2. CONCRETE SHALL ACHIEVE A MINIMUM OF 75% OF DESIGN STRENGTH BEFORE EXPANSION ANCHORS CAN BE INSTALLED.
- PA-3. ADHESIVE ANCHORS SHALL BE HILTI HIT-HY 200 (ICC ESR-3187), HIT-RE 500 V3 (ICC ESR-3814) ANCHOR SYSTEM, OR APPROVED EQUAL IN ACCORDANCE WITH ICC-ES AC308. FULLY THREADED RODS SHALL BE ASTM F1554 GRADE 55, MADE PER SUPPLEMENTARY REQUIREMENT S1, HOT-DIPPED GALVANIZED TO ASTM F2329, WITH ASTM A563 HEAVY HEX NUTS UNLESS NOTED OTHERWISE.
- PA-4. CONCRETE SHALL BE A MINIMUM OF 21 DAYS OLD BEFORE ADHESIVE ANCHORS CAN BE INSTALLED. PROOF TESTING SHALL BE REQUIRED AND COORDINATED WITH THE ENGINEER OF RECORD AND THE ADHESIVE MANUFACTURER FOR ANY ADHESIVE ANCHORS INSTALLED IN CONCRETE THAT IS LESS THAN 21 DAYS OLD.
- PA-5. EMBEDMENT SHALL BE AS ANNOTATED IN THE ANCHOR BOLT SCHEDULE.
- PA-6. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE DESIGN DRAWINGS. INSTALL ANCHORS TO MEET THE REQUIREMENTS INDICATED IN THE CONTRACT DOCUMENTS AND THE MANUFACTURER' S RECOMMENDATIONS. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S
- PA-7. FIELD PERSONNEL SHALL OBTAIN APPROVAL FROM THE ENGINEER OF RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS.
- PA-8. SPECIAL INSPECTION OF POST-INSTALLED ANCHORS SHALL BE PROVIDED AS REQUIRED BY ICC- ES EVALUATION REPORTS AND SECTION 1705.3 OF THE IBC AND ALL POST-INSTALLED ANCHOR INSTALLERS SHALL BE TRAINED BY THE MANUFACTURER'S FIELD REPRESENTATIVES.
- PA-9. FOLLOW MANUFACTURER INSTRUCTIONS FOR POST-INSTALLED ANCHORS INCLUDING BUT NOT LIMITED TO ANCHOR HOLE REQUIREMENTS.

SS STRUCTURAL STEEL

WRITTEN INSTRUCTIONS.

- SS-1. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE:
 - ..ASTM A992 a. W-SHAPES.. .ASTM A572 GRADE 50 b. L-SHAPES.. c. C-SHAPES.. .ASTM A572 GRADE 50 ..ASTM A500 GRADE C d. HSS.. e. SEAMLESS PIPE. ..ASTM A53 GRADE B f. PLATES i. UP TO 4" THICK, INCLUSIVE... ..ASTM A572 GRADE 50 ii. OVER 4" THICK.. ..ASTM A572 GRADE 50 g. SMOOTH RODS.
- SS-2. GUARDRAIL MEMBERS SHALL BE THE MATERIAL AND SIZE SHOWN BELOW FOR THE RESPECTIVE TYPE IN ORDER OF PREFERENCE:
- a. POST
- i. PIPE1-1/2XS... ..ASTM A53 GRADE B, TYPE E OR S ..ASTM A1085 OR ASTM A500 GRADE B/C ii. HSS1.900X0.188.

.ASTM A572 GRADE 50

..ASTM A53 GRADE B, TYPE E OR S

- b. SLEEVES .ASTM A53 GRADE B, TYPE E OR S i. PIPE2STD... ii. HSS2.375X0.154 ASTM A1085 OR ASTM A500 GRADE B/C
- c. OTHER MEMBERS

i. PIPE1-1/2STD..

BAR STOCK.

- ii. HSS1.900X0.145.. ASTM A1085 OR ASTM A500 GRADE B/C SS-3. WHERE NO CAMBER IS INDICATED, FABRICATE BEAMS SO THAT ANY NATURAL CAMBER
- IS UPWARD AFTER ERECTION.
- SS-4. SPLICES SHALL ONLY BE ALLOWED AT LOCATIONS SPECIFICALLY INDICATED ON THE DESIGN DRAWINGS UNLESS APPROVED OTHERWISE BY THE ENGINEER OF RECORD.
- SS-5. PROVIDE DRAIN HOLES IN ALL STEEL AS REQUIRED TO PREVENT ANY ACCUMULATION OF WATER. ALL PENETRATIONS THROUGH MAIN MEMBERS SHALL NOT EXCEED ONE INCH DIAMETER AND SHALL BE GROUND SMOOTH. DRAINS SHALL BE KEPT CLEAN AND
- SS-6. SHOW ALL COPES, HOLES, OPENINGS, AND MODIFICATIONS REQUIRED IN STRUCTURAL STEEL MEMBERS FOR ERECTION OR THE WORK OF OTHER TRADES ON THE SHOP DRAWINGS FOR APPROVAL BY THE ENGINEER OF RECORD.
- SS-7. FIELD MODIFICATION OF STRUCTURAL STEEL IS PROHIBITED WITHOUT PRIOR APPROVAL BY THE ENGINEER OF RECORD.
- SS-8. A QUALIFICATION TEST RECORD SHALL BE SUBMITTED FOR EACH WELDER ON SITE PERFORMING STRUCTURAL WELDING AS SHOWN ON THE DESIGN DRAWINGS.
- SS-9. WHERE MINIMUM CHARPY V-NOTCH IMPACT TESTING IS REQUIRED ON DESIGN DRAWINGS, SEE SPECIFICATION 93.62.02.
- SS-10. FOR HSS MEMBERS, 1/4" THICK CAP PLATES SHALL BE PROVIDED TO COVER ALL EXPOSED MEMBER ENDS. CAP PLATE DIMENSIONS SHALL BE 1/4" SMALLER THAN THE HSS EXTENTS AND SHALL BE ATTACHED USING A 1/8" ALL-AROUND FILLET WELD.

SC STRUCTURAL STEEL CONNECTIONS

- SC-1. ALL STEEL DETAILS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF AISC 360.
- SC-2. CONNECTION MATERIAL SHALL CONFORM TO THE FOLLOWING MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE:

ASTM F3125, GRADE A325, F1852, A490, OR F2280	a.	
ASTM A563	b.	
RSASTM F436	C.	
RSASTM F1554 GRADE 55, MADE PER S1	d.	

- ...COLD DRAWN CARBON STEEL BAR PER ASTM A29
- GRADES 1010 THRU 1020, ROUND 3/4" DIA, TYPE B HEADED STUD

- SC-3. ALL BOLTS CONNECTING STEEL TO STEEL SHALL FOLLOW THE AISC 348 SPECIFICATION.
- SC-4. ALL BOLTS SHALL BE TAKEN TO A SNUG-TIGHTENED CONDITION.
- SC-5. ALL BOLTS USED FOR PERMANENT BOLT-UP SHALL BE FREE OF DIRT AND RUST AND
- PROTECTED FROM THE WEATHER UNTIL INSTALLED. SC-6. A SKIDMORE WILHELM HYDRAULIC TENSION CALIBRATOR SHALL BE USED TO VERIFY
- THE PRETENSIONING METHOD DEVELOPS ADEQUATE BOLT PRETENSION AS REQUIRED SC-7. BOLT HOLES SHALL NOT BE REAMED OR DRILLED IN THE FIELD PRIOR TO RECEIVING
- APPROVAL FROM THE ENGINEER OF RECORD, EXCEPT BOLT HOLES MAY BE REAMED UP TO 1/32" LARGER THAN THEIR SPECIFIED DIAMETERS PER AISC 348 TABLE 3.1 USING THE APPROPRIATELY SIZED BRIDGE REAMER.
- SC-8. PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM AFTER WORK IS
- SC-9. EXCEPT WHERE SPECIFICALLY NOTED, CONNECTION DETAILS ON THE DRAWINGS ARE CONSIDERED COMPLETELY DESIGNED AND SHALL NOT BE MODIFIED WITHOUT SPECIFIC WRITTEN APPROVAL FROM THE ENGINEER OF RECORD. WHERE DETAILING. FABRICATION REQUIREMENTS, ERECTION REQUIREMENTS, OR FIELD PERSONNEL'S PREFERENCES REQUIRE MODIFICATIONS TO THESE CONNECTION DESIGNS. FABRICATOR SHALL CONSULT THE ENGINEER OF RECORD. ALTERNATIVE CONNECTIONS TO THOSE SHOWN ON DRAWINGS MAY ONLY BE CONSIDERED ACCEPTABLE IF THE FABRICATOR FORMALLY SUBMITS ALTERNATIVES AND THE ENGINEER OF RECORD APPROVES THE SUBMITTAL
- SC-10. FOR CONNECTION DESIGN AND DETAILING, SET CONNECTION WORK POINT AT INTERSECTION OF MEMBER CENTROIDS UNLESS NOTED OTHERWISE.
- SC-11. BEAM CONNECTION DESIGN NOTES:
 - a. BOLTED MOMENT CONNECTIONS AT CANTILEVERS AND BACK SPANS SHALL BE PRETENSIONED JOINTS.
 - b. DO NOT USE OVERSIZED OR SLOTTED HOLES FOR ANY CONNECTIONS UNLESS SPECIFICALLY INDICATED ON THE DRAWINGS OR APPROVED IN WRITING BY THE ENGINEER OF RECORD.
- SC-12. ALL WELDING SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.1. WELD PREPARATION AND WELD PROCESS SELECTION TO MEET THIS CRITERION IS THE RESPONSIBILITY OF THE FABRICATOR.
- SC-13. ALL WELD SIZES SHALL BE THE LARGER OF THE SIZE REQUIRED BY CONNECTION FORCES, THE MINIMUM SIZE PER AWS D1.1, OR 3/16" MINIMUM FILLET WELD UNLESS NOTED OTHERWISE. FILLET WELD SIZES SHOWN ON THE DESIGN DRAWINGS SHALL BE INCREASED IN ACCORDANCE WITH AWS D1.1 AS REQUIRED BY GAPS OR SKEWS BETWEEN COMPONENTS.
- SC-14. ALL PARTIAL JOINT PENETRATION WELD SIZES INDICATED ARE EFFECTIVE. WELD PREPARATION AND WELD PROCESS SELECTION TO MEET THIS CRITERION IS THE RESPONSIBILITY OF THE FABRICATOR.
- SC-15. USE RUNOFF TABS AT ALL BEVEL AND FULL PENETRATION WELDS. REMOVE RUNOFF TABS BY NEAT CUTS AFTER WELD IS COMPLETED. GRIND SMOOTH WHERE REQUIRED.
- SC-16. AT FULL PENETRATION WELDS, REMOVE WELD BACK UP BARS AND GRIND SMOOTH AFTER WELD IS COMPLETED UNLESS NOTED OTHERWISE.
- SC-17. ALL CONNECTIONS SHOWN ON THE DRAWINGS ARE DESIGNED UTILIZING THE FOLLOWING BOLT SIZES AND GRADES:
 - a. 7/8" DIAMETER ASTM F3125 GRADE A325/F1852 BOLTS UNLESS NOTED OTHERWISE
- SC-18. TYPICAL CONNECTION DIMENSIONS UNLESS NOTED OTHERWISE:

JC-10.		ICAL CONNECTION DIMENSIONS ONLESS NOTED OTHERWISE.	
	a.	BOLT SPACING	3"
	b.	MINIMUM EDGE DISTANCE	1 1/2"
	C.	HOLE TYPE	STANDARD
	d.	BEAM GAGE	STANDARD
	e.	ANGLE GAGE	STANDARD

- SC-19. PROVIDE 1/2" CLEAR DISTANCE BETWEEN CONNECTION ELEMENTS UNLESS NOTED
- SC-20. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR ERECTOR. BOLTS SHOWN GRAPHICALLY AS HEX BOLTS MAY BE TENSION CONTROL BOLTS AS PERMITTED BY THE GENERAL NOTES.
- SC-21. CONNECTION CALLOUT APPLIES TO BOTH ENDS OF MEMBER ON DESIGN DRAWINGS UNLESS NOTED OTHERWISE.
- SC-22. AT WELDED HSS-TO-HSS CONNECTIONS. WHERE AN ALL-AROUND FILLET WELD IS SPECIFIED ON THE DESIGN DRAWINGS, IT IS ACCEPTABLE TO SUBSTITUTE FLARE BEVEL GROOVE WELDS WHEN THERE IS INSUFFICIENT WELD SHELF TO ACHIEVE A FILLET WELD. FLARE BEVEL GROOVE WELD EFFECTIVE THROAT SHALL EQUAL 0.59 TIMES THE HSS WALL THICKNESS.

SD STEEL DECK GENERAL REQUIREMENTS

- SD-1. THE DESIGN, MANUFACTURE, AND ERECTION OF STEEL DECK AND ITS ANCHORAGE SHALL, AT A MINIMUM, BE IN ACCORDANCE WITH THE GOVERNING SDI STANDARD.
- SD-2. FABRICATE STEEL DECK UNITS AND ACCESSORIES FROM STEEL SHEET CONFORMING TO ASTM A653 SS GRADE 50 UNLESS NOTED OTHERWISE.
- SD-3. STEEL DECK SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A653. COATING DESIGNATION G90 UNLESS NOTED OTHERWISE.
- SD-4. CONFIGURE ALL STEEL DECK USING THREE SPAN CONTINUOUS LAYOUTS WHEREVER POSSIBLE UNLESS NOTED OTHERWISE.
- SD-5. CONFIGURE ALL STEEL DECK AS SHOWN ON THE DRAWINGS.
- SD-6. STEEL DECK HAS BEEN DESIGNED FOR UNSHORED CONDITIONS UNLESS NOTED OTHERWISE
- SD-7. COMPOSITE STEEL FLOOR DECK HAS BEEN DESIGNED FOR A MINIMUM CONSTRUCTION LIVE LOAD OF 50 PSF.
- SD-8. STEEL ROOF DECK HAS BEEN DESIGNED FOR A MINIMUM CONSTRUCTION LIVE LOAD OF
- SD-9. STEEL DECK SHALL NOT BE CANTILEVERED UNLESS SPECIFICALLY NOTED ON THE
- SD-10. FIELD PERSONNEL SHALL COORDINATE DECK OPENING SIZES AND LOCATIONS FROM ARCHITECTURAL AND MECHANICAL/ELECTRICAL/PLUMBING DRAWINGS. PROVIDE HEADER MEMBERS OR REINFORCEMENT AS REQUIRED BY TYPICAL DETAILS EVEN IF NOT SHOWN ON THE PLANS, AND SUBMIT PROPOSED OPENINGS THROUGH SLAB/DECK FOR REVIEW BY THE ENGINEER OF RECORD.
- SD-11. STEEL DECK DAMAGED BY IMPROPER STORAGE SHALL NOT BE USED IN CONSTRUCTION.
- SD-12. ALL STEEL DECK OPENINGS AND LEADING EDGES SHALL BE PROTECTED DURING CONSTRUCTION.

FD COMPOSITE AND NON-COMPOSITE STEEL FLOOR DECK

- FD-1. PROVIDE STEEL FLOOR DECK WITH THE DEPTH AND GAGE INDICATED ON THE DESIGN
- FD-2. DISTRIBUTE STEEL STUDS UNIFORMLY OVER BEAM SPAN UNLESS NOTED OTHERWISE. MAXIMUM SPACING OF HEADED STUDS SHALL NOT EXCEED 12" ON CENTER (ONE STUD
- FD-3. HEADED SHEAR STUDS SHALL BE 3/4" DIAMETER AND EXTEND A MINIMUM OF 1 1/2" ABOVE THE TOP OF STEEL DECK WITH A MINIMUM CLEAR COVER OF 1/2" FROM THE TOP
- FD-4. STEEL FLOOR DECK-SLABS SHALL BE POURED LEVEL AND CONCRETE FIELD PERSONNEL SHALL INCLUDE ADDITIONAL QUANTITY OF CONCRETE DUE TO BEAM AND DECK DEFLECTION, OR AS INDICATED ON THE DESIGN DRAWINGS.
- FD-5. DESIGN AND DETAIL DECK ENCLOSURES AND DECK ACCESSORIES FOR CONSTRUCTIONS LOADS.
- FD-6. DO NOT LOAD DECK UNTIL THE CONCRETE HAS ATTAINED 100% OF ITS DESIGN STRENGTH.

RD STEEL ROOF DECK

- RD-1. PROVIDE STEEL ROOF DECK WITH THE DEPTH AND GAGE INDICATED ON THE DESIGN DRAWINGS. PROVIDE ANCHORAGE TO SUPPORTING MEMBERS AS INDICATED ON THE DESIGN DRAWINGS.
- RD-2. DO NOT DIRECTLY HANG FROM STEEL ROOF DECK WITHOUT THE PRIOR APPROVAL FROM THE ENGINEER OF RECORD.
- RD-3. UNSCHEDULED ROOF OPENINGS SHALL BE REINFORCED PER THE APPROVAL OF THE ENGINEER OF RECORD.

ER STEEL ERECTION

- ER-1. ERECT STRUCTURAL STEEL PER AISC 303.
- ER-2. THE FIELD PERSONNEL SHALL COORDINATE A STEEL ERECTION PROCEDURE WITH THE ENGINEER OF RECORD SUBMIT AN ERECTION PROCEDURE PREPARED BY AN ENGINEER LICENSED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED FOR REVIEW BY THE ENGINEER OF RECORD. THIS PROCEDURE MUST INCLUDE THE SURVEY REQUIREMENTS AS DEFINED BY SPECIFICATION 95.62.
- ER-3. ALL WORK SHALL BE DONE IN ACCORDANCE WITH SPECIFICATION 95.62.

SG STEEL GRATING

- SG-1. ALL GRATING SHALL BE PLAIN TYPE 19-W-4 WITH 1 1/4" DEEP BY 3/16" THICK BEARING BARS UNLESS NOTED OTHERWISE. EXTERIOR GRATING SHALL BE SERRATED.
- SG-2. ALL GRATING AND MISCELLANEOUS PLATE STEEL SHALL BE GALVANIZED UNLESS NOTED OTHERWISE.
- SG-3. ALL GRATING SHALL BE ASTM A1011 STEEL. GRATING SHALL BE BANDED AT EDGES AND OPENINGS WITH BAR THE SAME SIZE AS THE BEARING BARS.
- SG-4. ALL GRATING SHALL BE SECURELY FASTENED TO SUPPORTING STEEL WITH CLIPS UNLESS NOTED OTHERWISE. FOLLOW MANUFACTURER RECOMMENDATIONS FOR GRATING ATTACHMENTS.
- SG-5. AT EACH END OF GRATING SPAN, PROVIDE 1" MINIMUM BEARING SURFACE FOR GRATING DEPTHS UP TO AND INCLUDING 2 1/4", AND 2" MINIMUM BEARING SURFACE FOR GRATING DEPTHS EXCEEDING 2 1/4".

CF COLD-FORMED STEEL FRAMING

- CF-1. ALL COLD-FORMED STEEL FRAMING ON STRUCTURAL DRAWINGS IS FOR DESIGN INTENT ONLY.FINAL DESIGN AND COORDINATION IS THE RESPONSIBILITY OF COLD-FORMED METAL FRAMING PROVIDER.
- CF-2. ALL COLD-FORMED STEEL FRAMING SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND IN ACCORDANCE
- CF-3. STEEL FOR ALL 14 AND 16 GAGE STUDS SHALL HAVE A MINIMUM YIELD STRENGTH OF 50 KSI. STEEL FOR ALL 20 GAGE STUDS AND FOR ALL GAGES OF TRACK, ACCESSORIES AND BRIDGING SHALL HAVE A MINIMUM YIELD STRENGTH OF 33 KSI.
- CF-4. ALL COLD-FORMED STEEL FRAMING SHALL BE GALVANIZED.
- CF-5. ALL STUDS SHALL BE SECURELY SEATED FOR FULL END BEARING ON TOP AND BOTTOM TRACKS UNLESS NOTED OTHERWISE.
- CF-6. PROVIDE DOUBLE STUDS AT ALL JAMB CORNERS, INTERSECTIONS, BEAM BEARINGS, AND JOIST BEARINGS.
- CF-7. BRIDGING SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS WITH THE FOLLOWING MINIMUM REQUIREMENTS: FOR NON-BEARING WALLS, PROVIDE BRIDGING AT MID-HEIGHT FOR WALLS LESS THAN OR EQUAL TO 10'-0" HIGH. PROVIDE BRIDGING AT 5'-0" ON CENTER MAXIMUM FOR WALLS GREATER THAN 10'-0" HIGH.
- CF-8. FIELD WELDING OF STEEL STUDS SHALL NOT BE PERMITTED WITHOUT PRIOR APPROVAL BY THE ENGINEER OF RECORD.
- CF-9. SUBMIT CALCULATIONS AND SHOP DRAWINGS, PREPARED BY A PROFESSIONAL ENGINEER LICENSED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED, FOR ALL COLD-FORMED STEEL FRAMING.

SA SAFETY AND ACCESS

- SA-1. GUARDRAIL DESIGNED TO MEET HEIGHT, SPACING, AND OTHER PROVISIONS IN ACCORDANCE WITH OSHA STANDARDS – 29 CFR, PART 1910, SUBPART D AND CAL/OSHA – TITLE 8.
- SA-2. ALL GUARDRAILS SHALL BE GALVANIZED OR PAINTED.
- SA-3. LADDERS AND CAGES SHALL HAVE ALL EDGES, CORNERS, AND WELDS GROUND
- SA-4. THE PERMANENT FALL PROTECTION SYSTEM AND ALL COMPONENTS SHALL COMPLY WITH OSHA STANDARDS - 29 CFR, PART 1910, SUBPART I (PERSONAL PROTECTIVE EQUIPMENT) AND APPLICABLE STATE STANDARDS FOR FALL RESTRAINT AND FALL
- SA-5. THE SYSTEM SHALL BE CAPABLE OF SUPPORTING A MINIMUM OF TWO WORKERS AT 5000 LB PER WORKER.
- SA-6. SUBMIT PRODUCT DATA, COMPONENT LIST, MAINTENANCE DATA AND TEST REPORTS DEMONSTRATING COMPLIANCE WITH CONTRACT REQUIREMENTS FOR REVIEW.
- SA-7. PROVIDE SOFTENERS AT ALL EDGES, BEAM FLANGES, CORNERS, ETC.
- SA-8. WIRE ROPE SHALL NOT BE USED WHERE AN ELECTRICAL HAZARD IS ANTICIPATED. SA-9. WIRE ROPE SHALL MEET THE REQUIREMENTS OF ASTM A492 STAINLESS STEEL ROPE
- SA-10. WIRE ROPE SHALL NOT BE LESS THAN 3/8" DIAMETER.

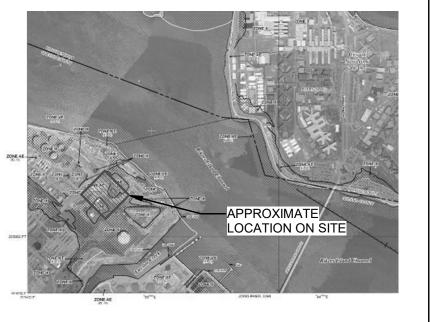
FL FLOOD

- FL-1. THE SITE IS DEFINED AS A ZONE (NON-COASTAL).
- BASE FLOOD ELEVATION, BFE, = 13 FEET NAVD 88.
- RECOMMENDED FREE BOARD BY THE 2022 NYCBC-APPENDIX G IS 2.0 FEET.
- FL-4. RECOMMENDED DESIGN FLOOD ELEVATION, DFE = 15 FEET NAVD 88.
- FL-5. THE STRUCTURE IS NOT SUBJECTED TO HYDRODYNAMIC LOADS.

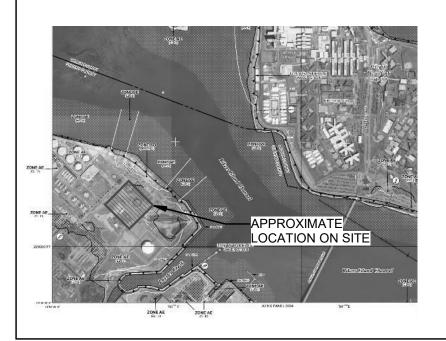
DESIGN ELEVATION / DEPTH	FEET IN NAVD 88
DESIGN BASE FLOOD ELEVATION	13.0
FINISH FLOOR ELEVATION	15.0
SEA LEVEL RISE	NOT CONSIDERED

FEMA FIRM PANEL 3604970092:

FLOOD INSURANCE RATE MAP 2007 **ELEVATIONS IN NAVD 88**



FLOOD INSURANCE RATE MAP 2015 **ELEVATIONS IN NAVD 88 FIRM IS PRELIMINARY**



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370 7th Avenue **SUITE 1604** New York, NY 10001



25 Mohawk Avenue **Sparta, NJ 07871**

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В	FINAL SUBMISSION	DJF	WA	12/12/2022
Α	INTERIM SUBMISSION	DJF	WA	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



901 Main Campus Drive Raleigh, North Carolina 27606



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

STRUCTURAL GENERAL **NOTES**

> 12/12/2022 PROJECT NO 105121 DRAWING BY D. FLYNN CHECKED BY W. ABBASSI

> > DRAWING NO

Autodesk Docs://CHPE Astoria/CHA-KIE-000-XX-M2-S-001.rvt

SI-1. REQUIRED SPECIAL INSPECTION OF STEEL CONSTRUCTION

	TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED	BC REFERENCED
1.	MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, N	L NUTS AND WASHERS:			
a.	IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	AISC 360, SECTION A3.3; APPLICABLE ASTM MATERIAL SPECIFICATIONS; AND RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH- STRENGTH BOLTS SECTION 2	-
b.	MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	X	RCSC SPECIFICATION FOR STRUCTURAL JOINTS USAGE HIGH-STRENGTH BOLTS SECTION 2.1	-
2.	INSPECTION OF HIGH-STRENGTH BOLTING:				
a.	SNUG-TIGHT JOINTS.	-	х		
b.	PRE TENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITH MATCHMARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION.	-	X	AISC 360 SECTION M2.5; AND RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING	1705.2.3
C.	PRE-TENSIONED AND SLIP CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING OR CALIBRATED WRENCH METHODS OF INSTALLATION.	X	-	HIGH-STRENGTH BOLTS SECTION 9	
d.	PRE-INSTALLATION VERIFICATION TESTING.	X	-	SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS SECTION 8.2	1705.2.3.1
3.	MATERIAL VERIFICATION OF STRUCTURAL STEEL AND	COLD FORMED STEEL	DECK:		
а.	FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.	-	Х	AISC 360 SECTIONS 43.1, N2.1, N3.2 (a) AND (k)(1)	-
b.	FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	-	APPLICABLE ASTM STANDARDS	-
C.	MANUFACTURERS' CERTIFIED MILL TEST REPORTS.	-	Х	APPLICABLE ASTM MATERIAL STANDARDS	
4.	MATERIAL VERIFICATION OF WELD FILLER MATERIALS:				
a.	IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS	-	-	AISC 360 SECTIONS 43.5 AND N3.2(e), AND APPLICABLE AWS A5 DOCUMENTS AND AWS D1.1 5.3.1 AND APPROVED CONTRACT DOCUMENTS	-
b.	MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	-	AISC 360 SECTION 43.5	-
5.	INSPECTION OF WELDING:				
a.	STRUCUTRAL STEEL:	-	-	-	-
	1) COMPLETE AND PARTIAL PENETRATION GROOVE WELDS.	Х	-		
	2) MULTIPASS FILLET WELDS.	X	-		
	3) SINGLE-PASS FILLET WELDS > 5/16".	Х	-	AWS D1.1	1705.2.1
	4) PLUG AND SLOT WELDS.	X	-		
	5) SINGLE-PASS FILLET WELDS > 5/16".	-	X		
	5) SINGLE-PASS FILLET WELDS > 5/16".6) FLOOR AND ROOF DECK WELDS.	-	×	AWS D1.3	-
	,	-		AWS D1.3	-
b.	6) FLOOR AND ROOF DECK WELDS.	- - -	X		-
b.	6) FLOOR AND ROOF DECK WELDS. 7) COLD FORMED STEEL WELDS.	- - -	X		-
b.	6) FLOOR AND ROOF DECK WELDS. 7) COLD FORMED STEEL WELDS. REINFORCING STEEL:	X	X X -		1903.6.2
b.	6) FLOOR AND ROOF DECK WELDS. 7) COLD FORMED STEEL WELDS. REINFORCING STEEL: 1) PRE-WELDING VERIFICATION OF BASE METAL. 2) REINFORCING STEEL-RESTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR	X NOTE a	X X -	AWS D1.3	1903.6.2

6.	INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS:			-	1705.2.2
a.	DETAILS SUCH AS BRACING AND STIFFENING.	-	X		
b.	MEMBER LOCATIONS.	-	X		
C.	APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	-	Х		

SI-2. REQUIRED SPECIAL INSPECTION OF COLD-FORMED STEEL CONSTRUCTION

	TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED	BC REFERENCE
1.	MATERIAL VERIFICATION:				
a.	VERIFY IDENTIFICATION MARKINGS CONFORM TO AISI S240 AND AS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	X	-	AISI S240, SECTION D6.5	-
b.	VERIFY THAT MATERIAL IS CLEAN, STRAIGHT AND UNDAMAGED.	-	X	-	-
2.	INSPECTION OF GENERAL FRAMING:				
a.	VERIFY THAT MEMBER SIZES CONFORM TO THE APPROVED CONSTRUCTION DOCUMENTS>	-	Х		-
b.	VERIFY THAT MEMBER LAYOUT CONFORMS TO THE APPROVED CONSTRUCTION DOCUMENTS.	-	X		-
C.	VERIFY THAT PROPER BEARING LENGTHS ARE PROVIDED IN ACCORDANCE WITH APPROVED CONSTRUCTION DOCUMENTS.	-	X		-
d.	VERIFY THAT PUNCHED HOLES AND SHEARED OR FLAME CUT EDGES OF MATERIAL IN MEMBERS ARE CLEAN AND FREE FROM NOTCHES AND BURRED EDGES.	-	Х	AISI S240 SECTION C	-
d.	PRE-INSTALLATION VERIFICATION TESTING.	X	-		1705.2.3.1
3.	INSPECTION OF FRAMING CONNECTIONS AND ANCHOR	RAGES:			
a.	VERIFY THAT SCREWS, BOLTS, AND OTHER FASTENERS CONFORM TO APPROVED CONSTRUCTION DOCUMENTS REQUIREMENTS FOR DIAMETER, LENGTH, QUANTITY, SPACING EDGE DISTANCE, AND LOCATIONS.	-	Х	AISI S240, SECTION D6.7	-
b.	VERIFY THAT MANUFACTURED CONNECTORS, SUCH AS JOIST HANGERS, CAPS, STRAPS, CLIPS, TIES, HOLD-DOWNS, AND ANCHORS CONFORM TO APPROVED CONSTRUCTION DOCUMENT REQUIREMENTS FOR MANUFACTURER, TYPE, GAUGE, AND FASTENER REQUIREMENTS.	-	-	AISI S240 SECTION D6.9	-
c.	POST-INSTALLED CONNECTIONS TO CONCRETE.	Х	-	AISI S240 SECTION D6.9	-
4.	INSPECTION OF WELDING:				
a.	INSPECT WELDS IN ACCORDANCE WITH S240 SECTION D6.6.	-	Х	AWS D1.3, AISI S240 SECTION D6.6	-
b.	ADDITIONAL REQUIREMENTS FOR WELDS PERFORMED AS A PART OF A LATERAL FORCE- RESISTING SYSTEM	X NOTE a	-	AISI S240 SECTION D6.9	-
5.	BRACING:				
	VERIFY THAT TEMPORARY BRACING, SHORING, JACKS, ETC., ARE INSTALLED, AND NOT REMOVED UNTIL NO LONGER NECESSARY, IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND APPROVED ERECTION DRAWINGS.	-	Х		-
BR WI AN	VERIFY THAT PERMANENT BRACING, WEB STIFFENERS, BRIDGING, BLOCKING, WIND ACING, ETC, ARE INSTALLED IN ACCORDANCE TH THE APPROVED CONSTRUCTION DOCUMENTS ID APPROVED ERECTION DRAWINGS.	-	Х	AISI S240 SECTION E6	-
с.	WHERE A COLD-FORMED STEEL TRUSS CLEAR SPAN IS 60 FEET (18 288 MM) OR GREATER, THE SPECIAL INSPECTOR SHALL VERIFY THAT THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE.	-	X		2211.1.3.2
6.	PRE-INSTALLATION DOCUMENT SUBMITTALS	-	Х	AISI S240, SECTION D3	-
7	LATERAL FORCE-RESISTING SYSTEM ADDITIONAL	_	X	AISI S240, SECTION D6.9	

SI-3. INSPECTORS SHALL SUBMIT REPORTS TO FIELD PERSONNEL AND ENGINEER OF RECORD INDICATING APPROVAL OF MATERIALS, METHODS OF CONSTRUCTION, AND COMPLIANCE WITH SPECIFICATIONS AFTER SATISFACTORY COMPLETION OF REQUIRED TESTS AND SUBMISSION OF REQUIRED TEST REPORTS.

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25 Mohawk Avenue **Sparta, NJ 07871**

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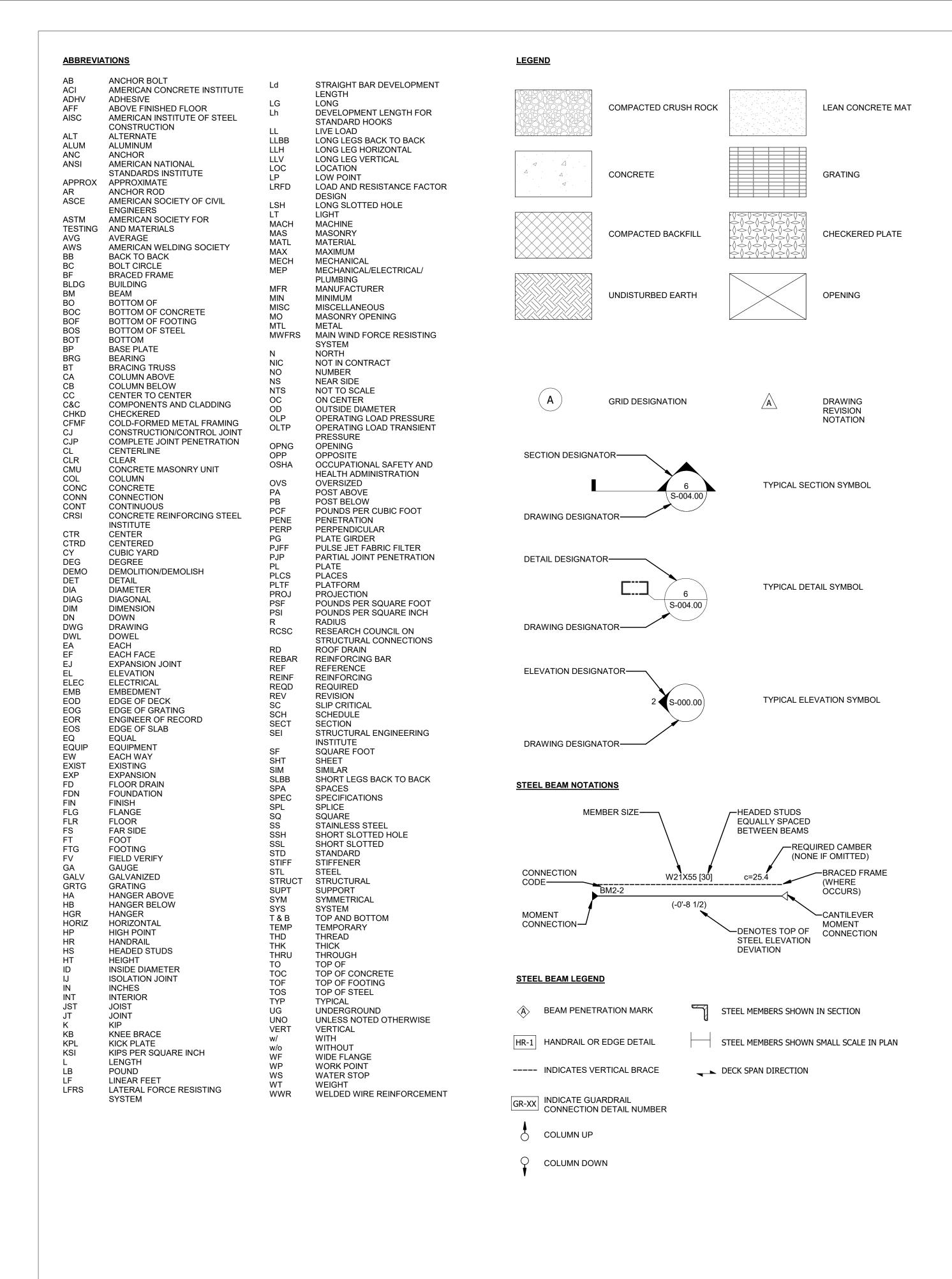
Astoria HVDC Converter Station

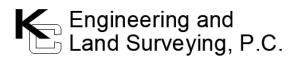
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STRUCTURAL GENERAL **NOTES**

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
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S-003.00





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REV	DESCRIPTION	DRW BY	CHK BY	DATE



@Hitachi Energy 901 Main Campus Drive Raleigh, North Carolina 27606

PROJECT



Astoria HVDC Converter Station

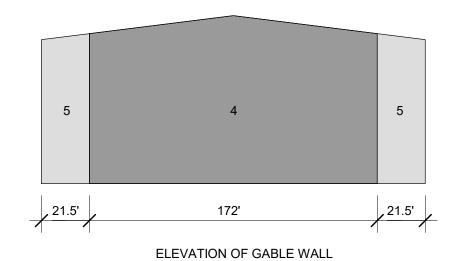
31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

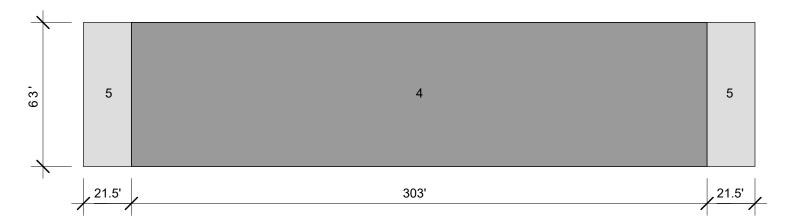
STRUCTURAL GENERAL **NOTES**

DATE 12/12/2022 PROJECT NO 105121 DRAWING BY D. FLYNN CHECKED BY W. ABBASSI DRAWING NO S-004.00

Autodesk Docs://CHPE Astoria/CHA-KIE-000-XX-M2-S-001.rvt

CONVERTER BUILDING C&C WIND LOAD

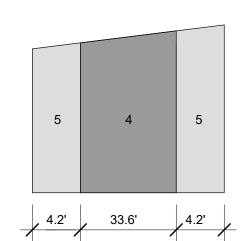




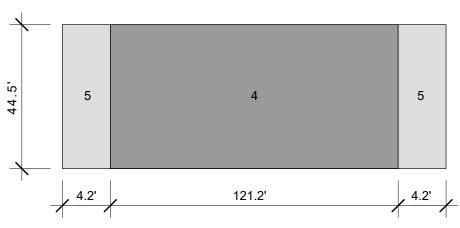
ELEVATION OF SIDE WALL

COMPONENT	ZONE	PRES (+VE) (PSF)	PRES (-VE) (PSF)
<=10 SF (W)	4	46.7	-46.7
50 SF (W)	4	43.0	-44.3
200 SF (W)	4	37.5	-40.5
>500 SF (W)	4	33.8	-33.8
<=10 SF (W)	5	46.7	-35.7
50 SF (W)	5	43.0	-75.8
200 SF (W)	5	37.5	-60.9
>500 SF (W)	5	33.8	-51.1

SERVICE BUILDING C&C WIND LOAD

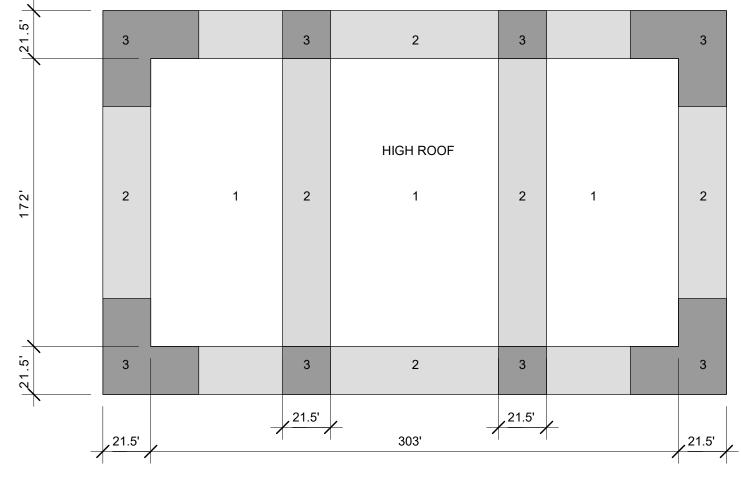


ELEVATION OF GABLE WALL



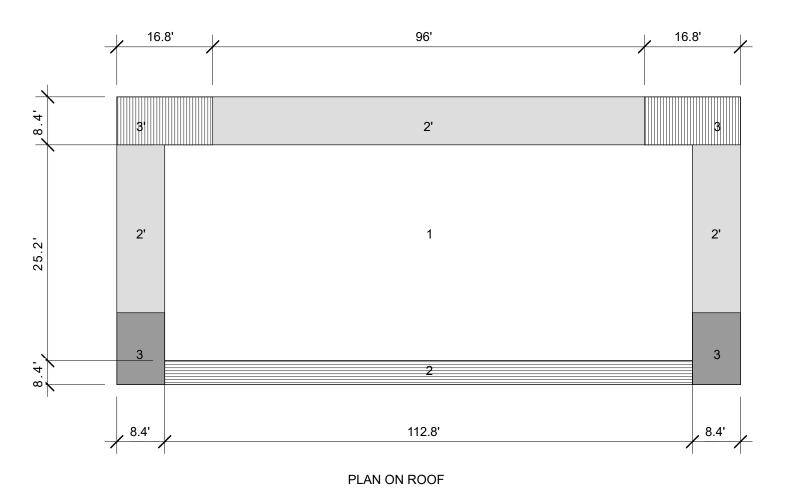
ELEVATION OF SIDE WALL

COMPONENT	ZONE	PRES (+VE) (PSF)	PRES (-VE) (PSF)
<=10 SF	4	43.0	-46.6
50 SF	4	38.6	-42.1
200 SF	4	34.7	-38.3
>500 SF	4	32.2	-35.8
<=10 SF	5	43.0	-57.3
50 SF	5	38.6	-48.5
200 SF	5	34.7	-40.8
>500 SF	5	32.2	-35.8



PLAN ON ROOF

COMPONENT	ZONE	PRES (-VE) (PSF)
<=10 SF	1	-68.4
20 SF	1	-64.5
50 SF	1	-59.5
>100 SF	1	-55.6
<=10 SF	2	-107.3
20 SF	2	-102.0
50 SF	2	-94.9
>100 SF	2	-89.5
<=10 SF	3	-146.3
20 SF	3	-139.4
50 SF	3	-130.2
>100 SF	3	-123.3



COMPONENT	ZONE	PRES (+VE) (PSF)	PRES (-VE) (PSF)
<=10 SF	1	19.1	-50.9
20 SF	1	17.9	-50.9
50 SF	1	16.3	-50.9
>100 SF	1	15.1 #	-50.9
<=10 SF	2	19.1	-58.9
20 SF	2	17.9	-57.7
50 SF	2	16.3	-56.1
>100 SF	2	15.1 #	-54.9
<=10 SF	2'	19.1	-70.8
20 SF	2'	17.9	-69.6
50 SF	2'	16.3	-68.0
>100 SF	2'	15.1 #	-66.8
<=10 SF	3	19.1	-78.8
20 SF	3	17.9	-71.6
50 SF	3	16.3	-62.1
>100 SF	3	15.1 #	-54.9
<=10 SF	3'	19.1	-110.6
20 SF	3'	17.9	-98.6
50 SF	3'	16.3	-82.8
>100 SF	3'	15.1 #	-70.8

THE FINAL NET DESIGN WIND PRESSURE, INCLUDING ALL PERMITTED REDUCTIONS, USED IN THE DESIGN SHALL NOT BE LESS THAN 16 PSF ACTING IN EITHER DIRECTION

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Α	FINAL SUBMISSION	DJF	WA	12/12/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



Hitachi Energy901 Main Campus Drive Raleigh, North Carolina 27606

PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

COMPONENTS AND CLADDING WIND LOAD DIAGRAMS

 DATE
 12/12/2022

 PROJECT NO
 105121

 DRAWING BY
 D. FLYNN

 CHECKED BY
 W. ABBASSI

 DRAWING NO

S-005.00

CADD FILE NO Autodesk Docs://CHPE



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Α	FINAL SUBMISSION	DJF	WA	12/12/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE
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901 Main Campus Drive Raleigh, North Carolina 27606



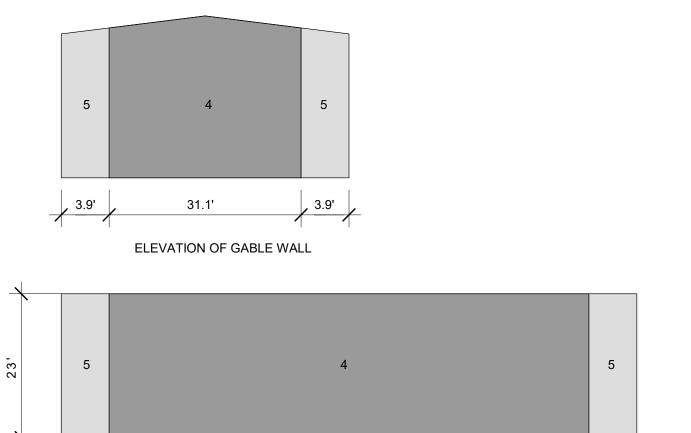
Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

COMPONENTS AND CLADDING WIND LOAD DIAGRAMS

DRAWING NO	
CHECKED BY	W. ABBASSI
DRAWING BY	D. FLYNN
PROJECT NO	105121
DATE	12/12/2022

S-006.00



ELEVATION OF SIDE WALL

COMPONENT	ZONE	PRES (+VE) (PSF)	PRES (-VE) (PSF)
<=10 SF	4	37.8	-41.0
50 SF	4	33.9	-37.1
200 SF	4	30.6	-33.7
>500 SF	4	28.4	-31.5
<=10 SF	5	37.8	-50.4
50 SF	5	33.9	-42.6
200 SF	5	30.6	-35.9
>500 SF	5	28.4	-31.5

7	/	12'-0"	
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11.2.		1	
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	3		3
	13.8"	84.3'	13.8"

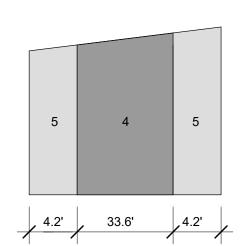
PLAN ON ROOF

COMPONENT	ZONE	PRES (+VE) (PSF)	PRES (-VE) (PSF)
<=10 SF	1	16.8	-65.8
100 SF	1	13.3#	-51.4
200 SF	1	13.3#	-47.1
>500SF	1	13.3#	-41.3
<=10 SF	2	16.8	-86.8
100 SF	2	13.3#	-68.3
200 SF	2	13.3#	-62.7
>500 SF	2	13.3#	-55.3
<=10 SF	3	16.8	-118.3
100 SF	3	13.3#	-81.3
200 SF	3	13.3#	-70.1
>500 SF	3	13.3 #	-55.3

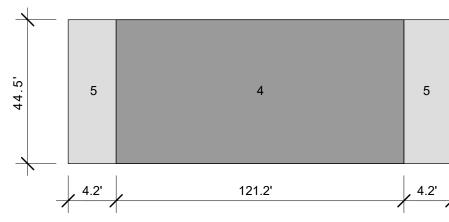
THE FINAL NET DESIGN WIND PRESSURE, INCLUDING ALL PERMITTED REDUCTIONS, USED IN THE DESIGN SHALL NOT BE LESS THAN 16 PSF ACTING IN EITHER DIRECTION

ALL ENCLOSURES WITH H</= 15'-0" C&C WIND LOAD

STORAGE ENCLOSURE C&C WIND LOAD

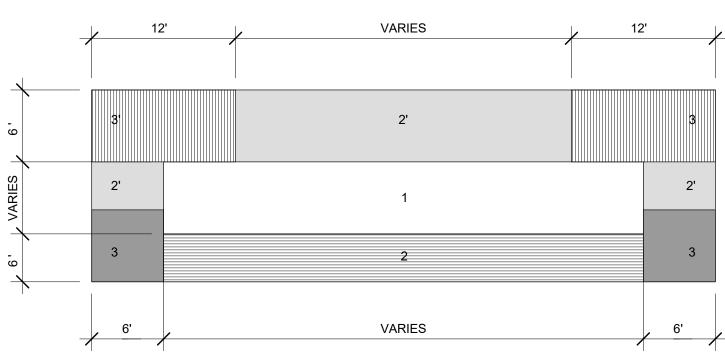


ELEVATION OF GABLE WALL



ELEVATION OF SIDE WALL

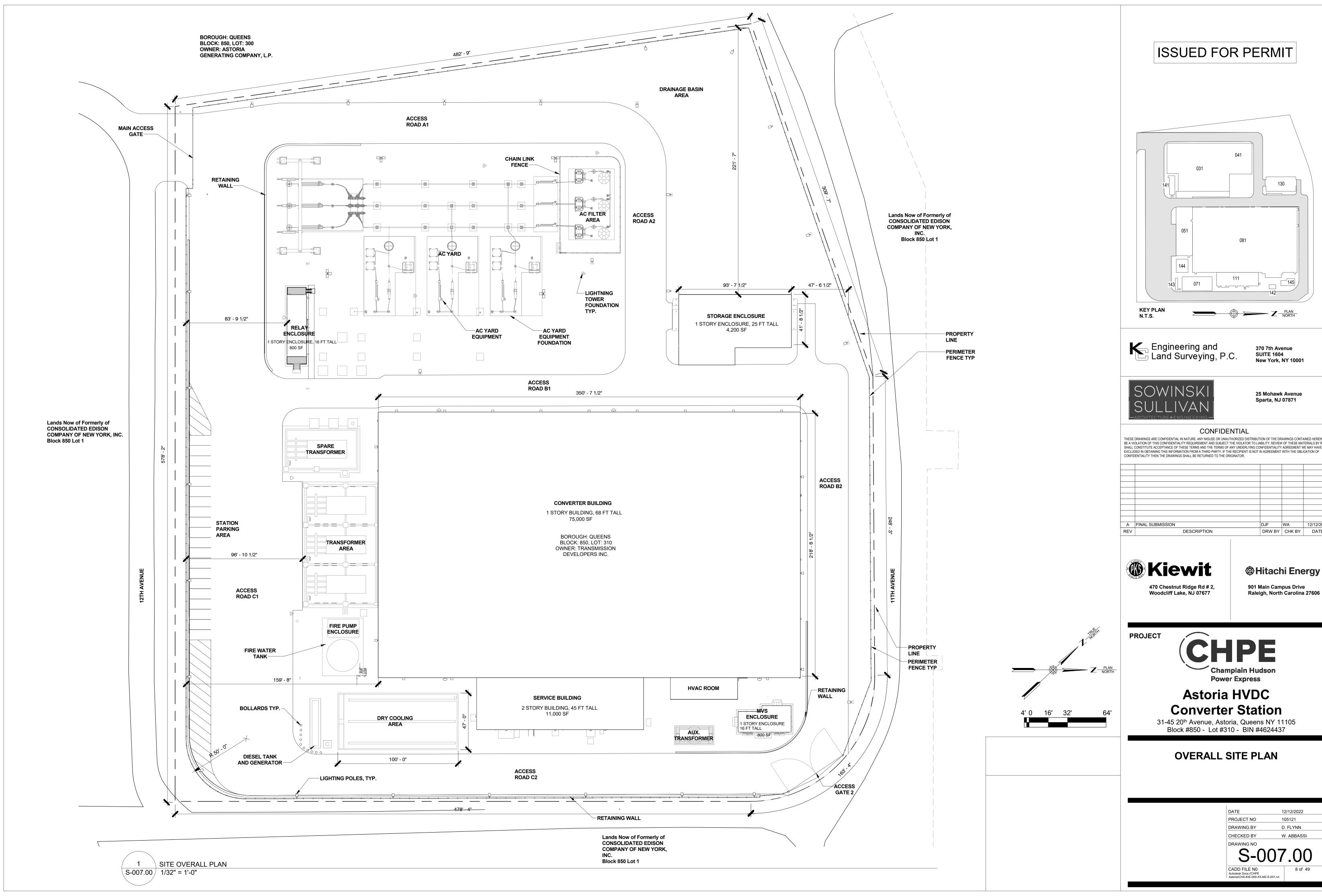
COMPONENT	ZONE	PRES (+VE) (PSF)	PRES (-VE) (PSF)
<=10 SF	4	34.8	-37.7
50 SF	4	31.2	-34.1
200 SF	4	28.1	-31.0
>500 SF	4	26.1	-29.0
<=10 SF	5	34.8	-46.4
50 SF	5	31.2	-39.2
200 SF	5	28.1	-33.1
>500 SF	5	26.1	-29.0

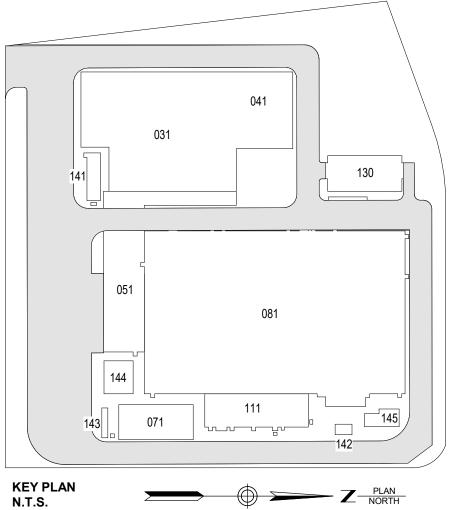


PLAN ON ROOF

COMPONENT	ZONE	PRES (+VE) (PSF)	PRES (-VE) (PSF)
<=10 SF	1	15.5 #	-41.2
20 SF	1	14.5 #	-41.2
50 SF	1	13.2 #	-41.2
>100 SF	1	12.2 #	-41.2
<=10 SF	2	15.5#	-47.7
20 SF	2	14.5#	-46.7
50 SF	2	13.2 #	-45.4
>100 SF	2	12.2 #	-44.4
<=10 SF	2'	15.5#	-57.3
20 SF	2'	14.5#	-56.4
50 SF	2'	13.2 #	-55.1
>100 SF	2'	12.2 #	-54.1
<=10 SF	3	15.5#	-63.8
20 SF	3	14.5#	-58.0
50 SF	3	13.2 #	-50.3
>100 SF	3	12.2 #	-44.4
<=10 SF	3'	15.5 #	-89.5
20 SF	3'	14.5 #	-79.8
50 SF	3'	13.2 #	-67.0
>100 SF	3'	12.2 #	-57.3

THE FINAL NET DESIGN WIND PRESSURE, INCLUDING ALL PERMITTED REDUCTIONS, USED IN THE DESIGN SHALL NOT BE LESS THAN 16 PSF ACTING IN EITHER DIRECTION

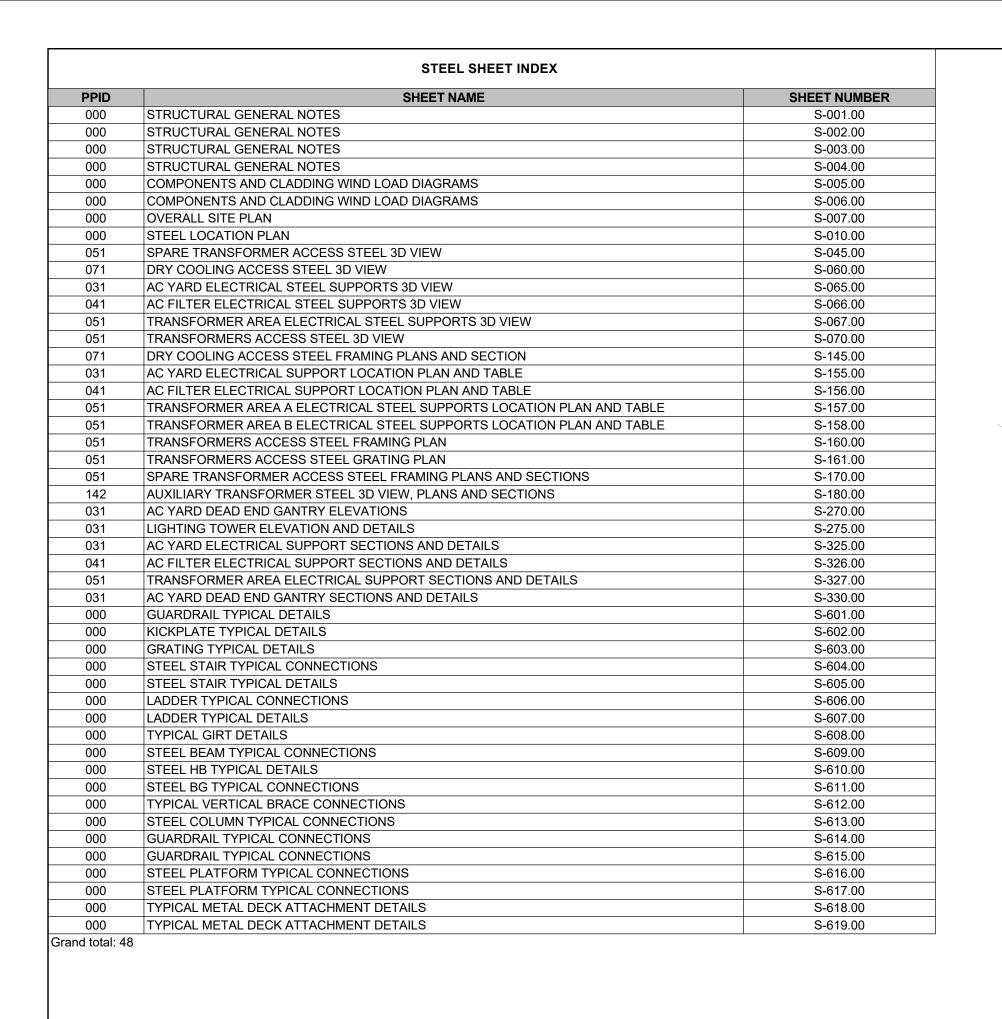


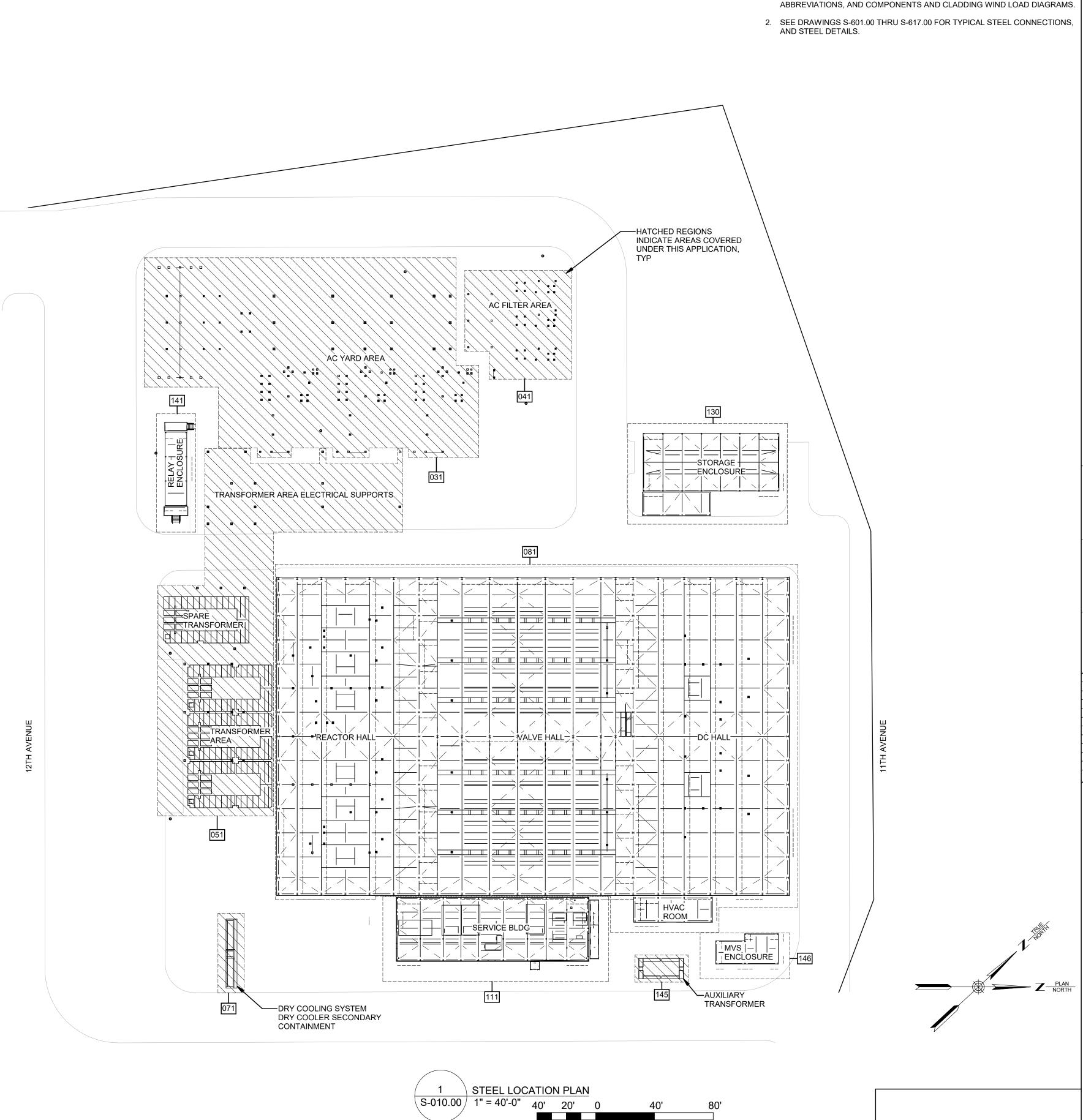


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	FINAL SUBMISSION	DJF	WA	12/12/2022
/	DESCRIPTION	DRW BY	CHK BY	DATE

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	

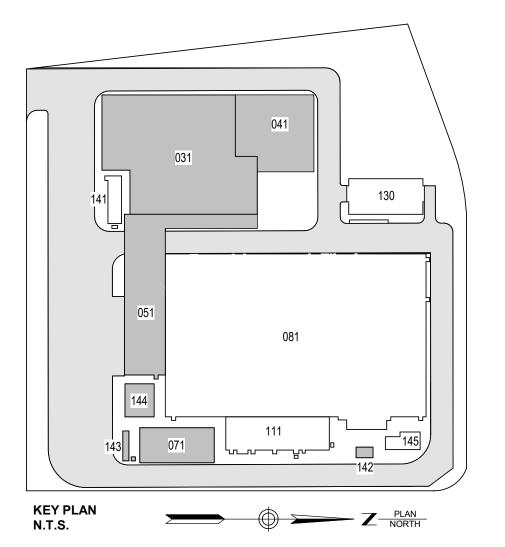




STRUCTURE NOTES:

1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND,





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В	FINAL SUBMISSION	DJF	WA	12/12/2022
Α	INTERIM SUBMISSION	DJF	WA	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



Hitachi Energy
 Main Campus Drive

Raleigh, North Carolina 27606

PROJECT



Astoria HVDC Converter Station

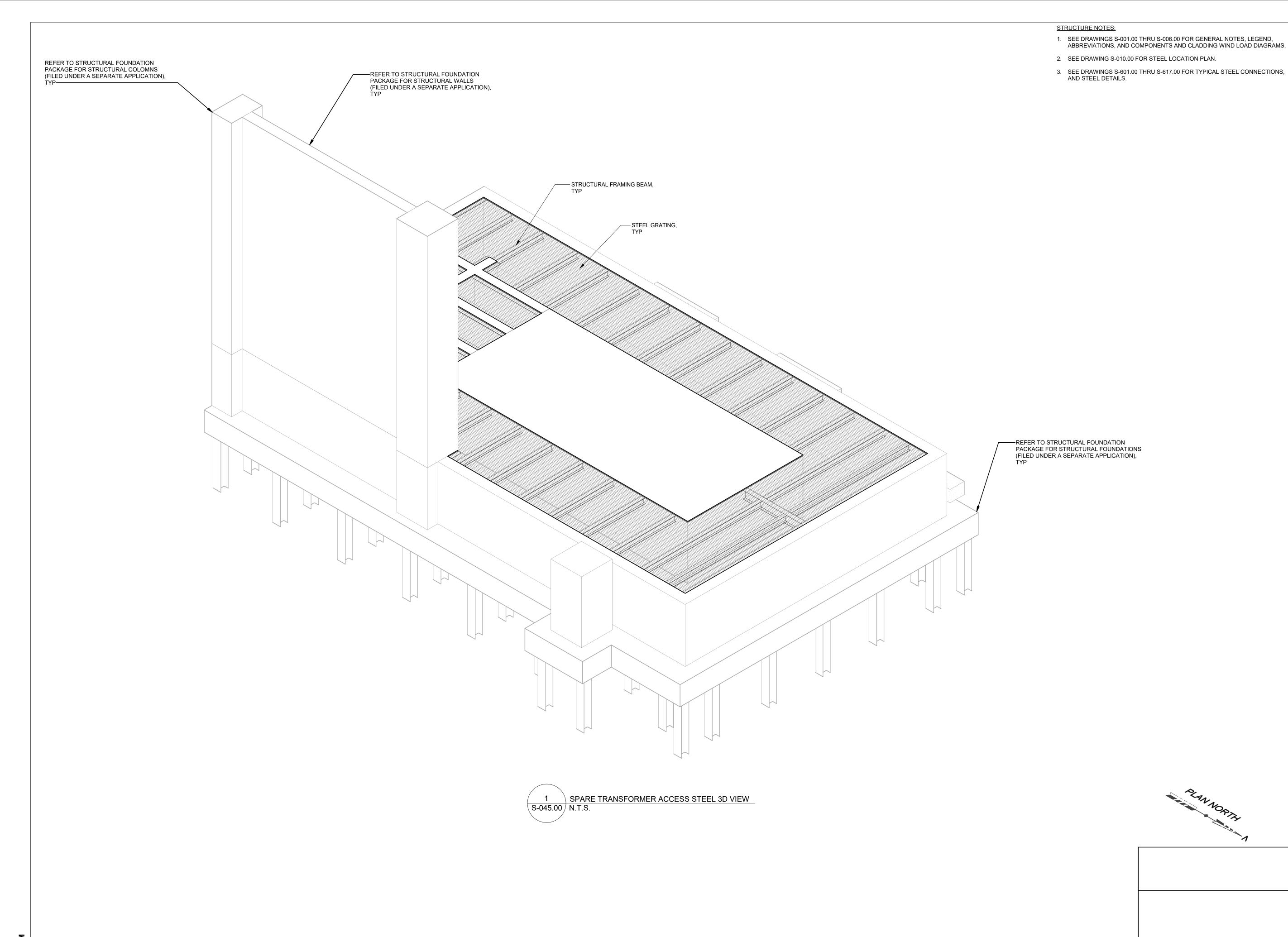
31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

STEEL LOCATION PLAN

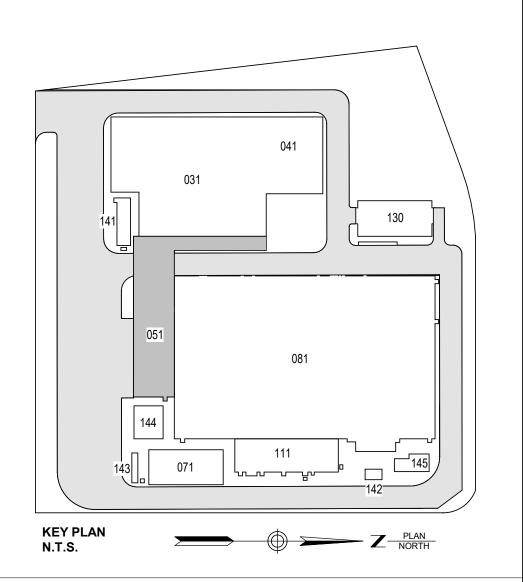
DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
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S-010.00

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В	FINAL SUBMISSION	DJF	DS	12/12/2022
Α	INTERIM SUBMISSION	DJF	BZ	09/13/2022
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Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

SPARE TRANSFORMER ACCESS STEEL 3D VIEW

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
	4 - 00

S-045.00

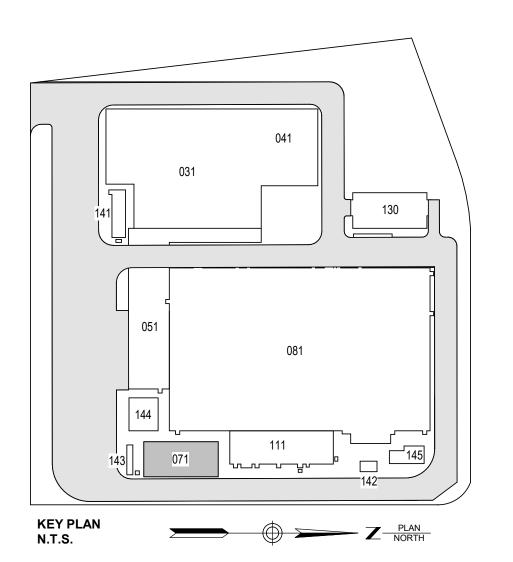
SPARE TRANSFORMER ACCESS STEEL SHEET INDEX

SHEET NAME
SPARE TRANSFORMER ACCESS STEEL 3D VIEW
SPARE TRANSFORMER ACCESS STEEL FRAMING PLANS AND SECTIONS
S-170.00

STRUCTURE NOTES:

- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. SEE DRAWING S-010.00 FOR STEEL LOCATION PLAN.
- 3. SEE DRAWINGS S-601.00 THRU S-617.00 FOR TYPICAL STEEL CONNECTIONS, AND STEEL DETAILS.

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В	FINAL SUBMISSION	DJF	DS	12/12/2022
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Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

DRY COOLING ACCESS STEEL 3D VIEW

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
\circ	\cap

S-060.00

DRYCOOLER ACCESS STEEL SHEET INDEX

SHEET NUMBER

DRY COOLING ACCESS STEEL FRAMING PLANS AND SECTION
DRY COOLING ACCESS STEEL 3D VIEW

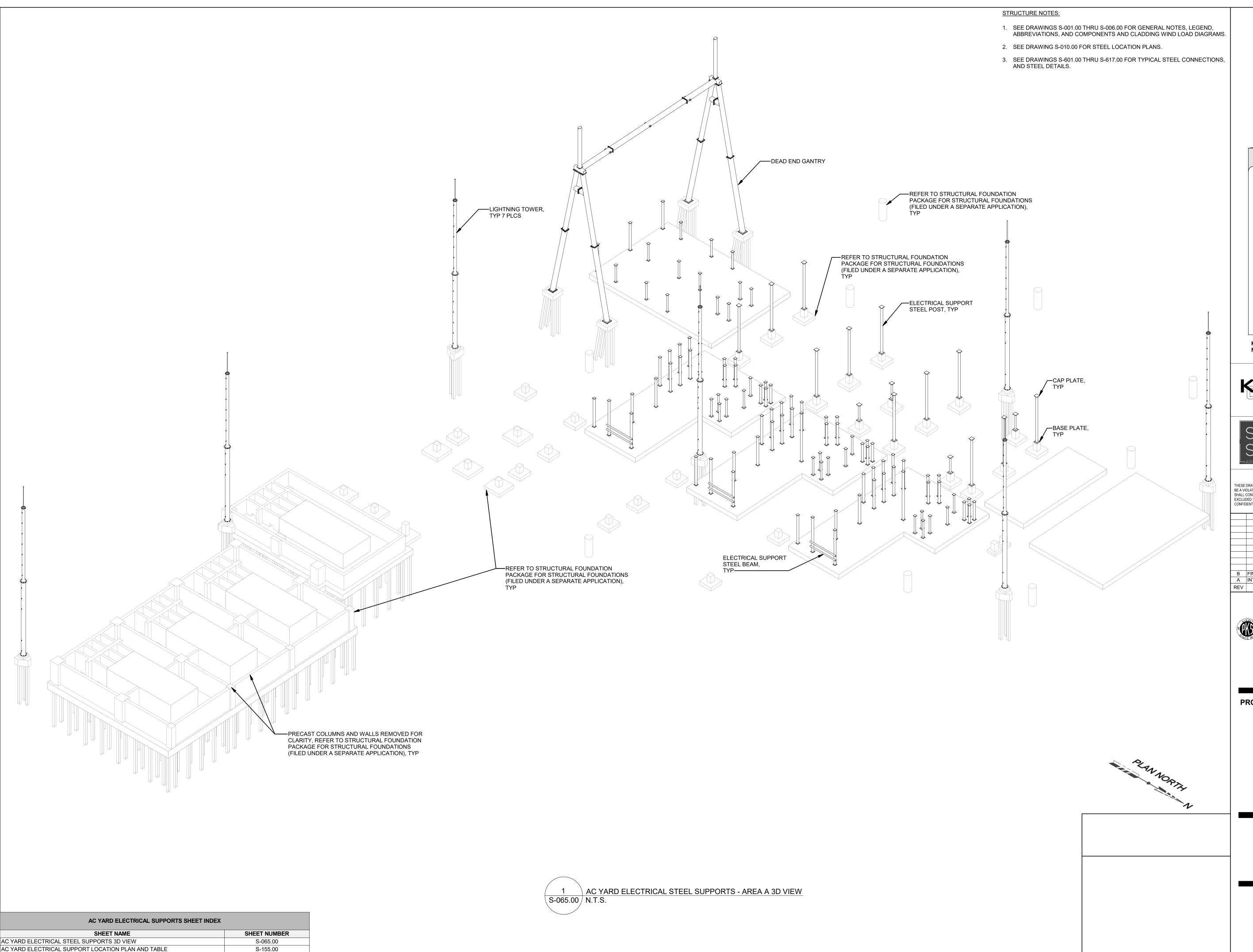
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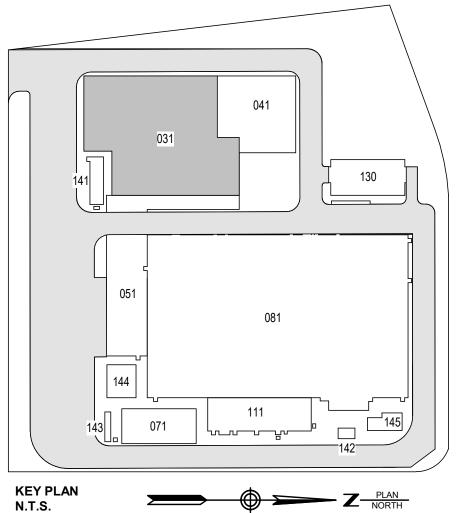
REFER TO STRUCTURAL FOUNDATION PACKAGE FOR STRUCTURAL FOUNDATIONS (FILED UNDER A SEPARATE APPLICATION),

STEEL GRATING,



PLANNORTH





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FINAL SUBMISSION	DJF	AA	12/12/2022
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DESCRIPTION	DRW BY	CHK BY	DATE
	FINAL SUBMISSION INTERIM SUBMISSION DESCRIPTION	INTERIM SUBMISSION GDB	INTERIM SUBMISSION GDB DS

470 Chestnut Ridge Rd # 2, Woodcliff Lake, NJ 07677

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PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

AC YARD ELECTRICAL STEEL SUPPORTS 3D VIEW

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	

S-065.00

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AC YARD DEAD END GANTRY ELEVATIONS

LIGHTNING TOWER ELEVATION AND DETAILS

AC YARD ELECTRICAL SUPPORT SECTIONS AND DETAILS

AC YARD DEAD END GANTRY SECTIONS AND DETAILS

S-270.00

S-275.00

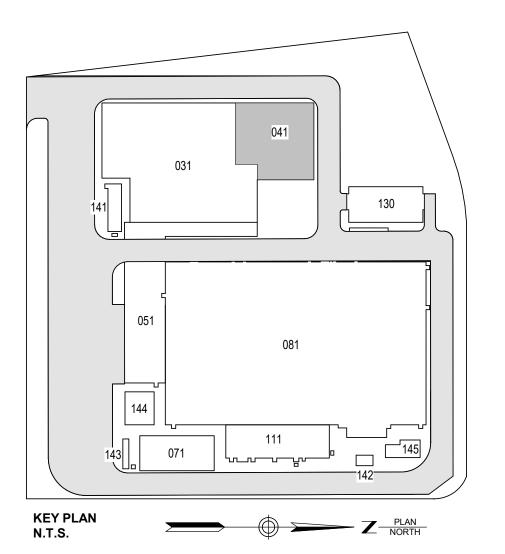
S-325.00

S-330.00

STRUCTURE NOTES:

- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. SEE DRAWING S-010.00 FOR STEEL LOCATION PLANS.
- 3. SEE DRAWINGS S-601.00 THRU S-617.00 FOR TYPICAL STEEL CONNECTIONS, AND STEEL DETAILS..

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В	FINAL SUBMISSION	DJF	AA	12/12/2022
Α	INTERIM SUBMISSION	GDB	DS	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE
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PROJECT



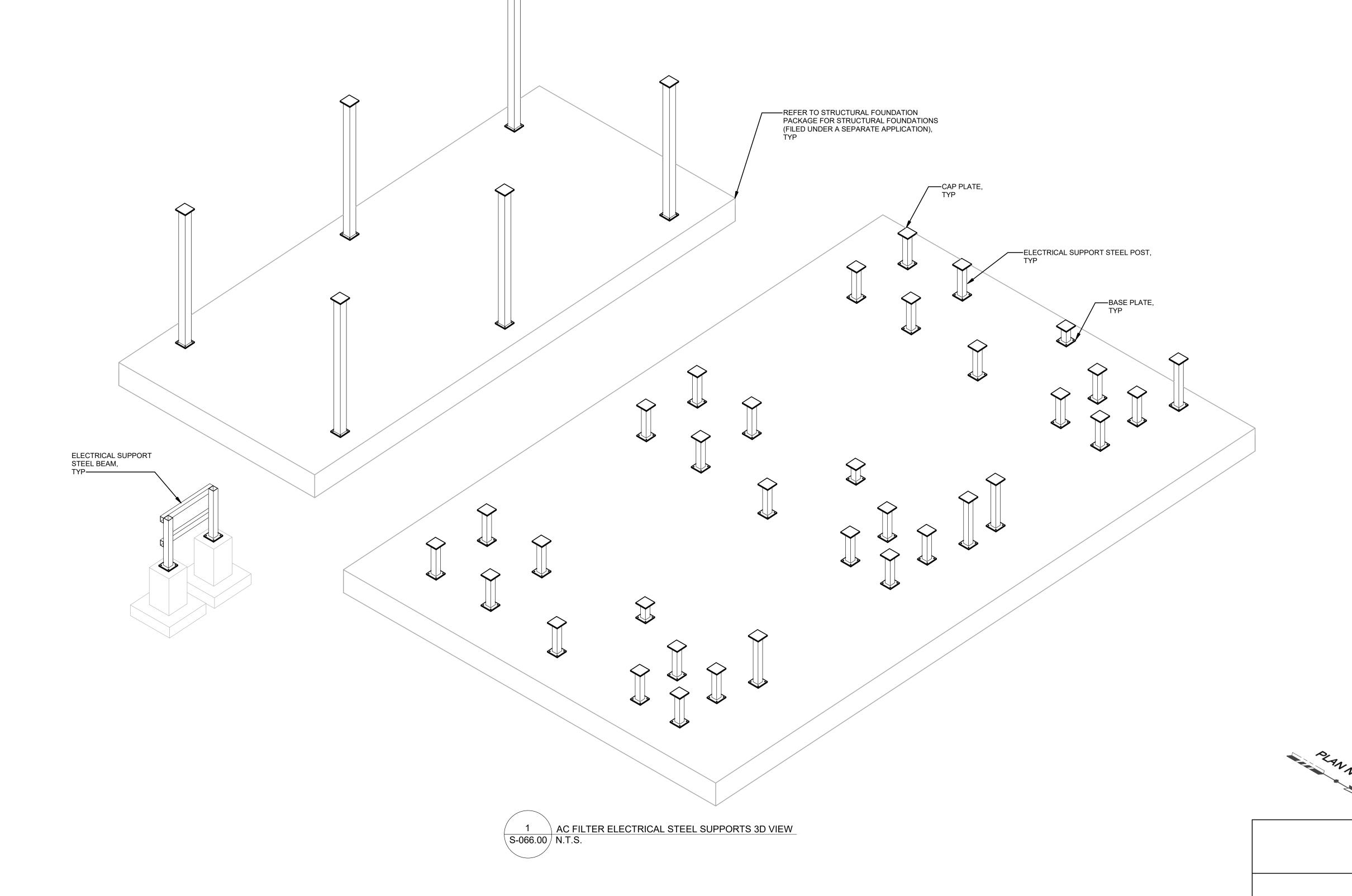
Astoria HVDC Converter Station

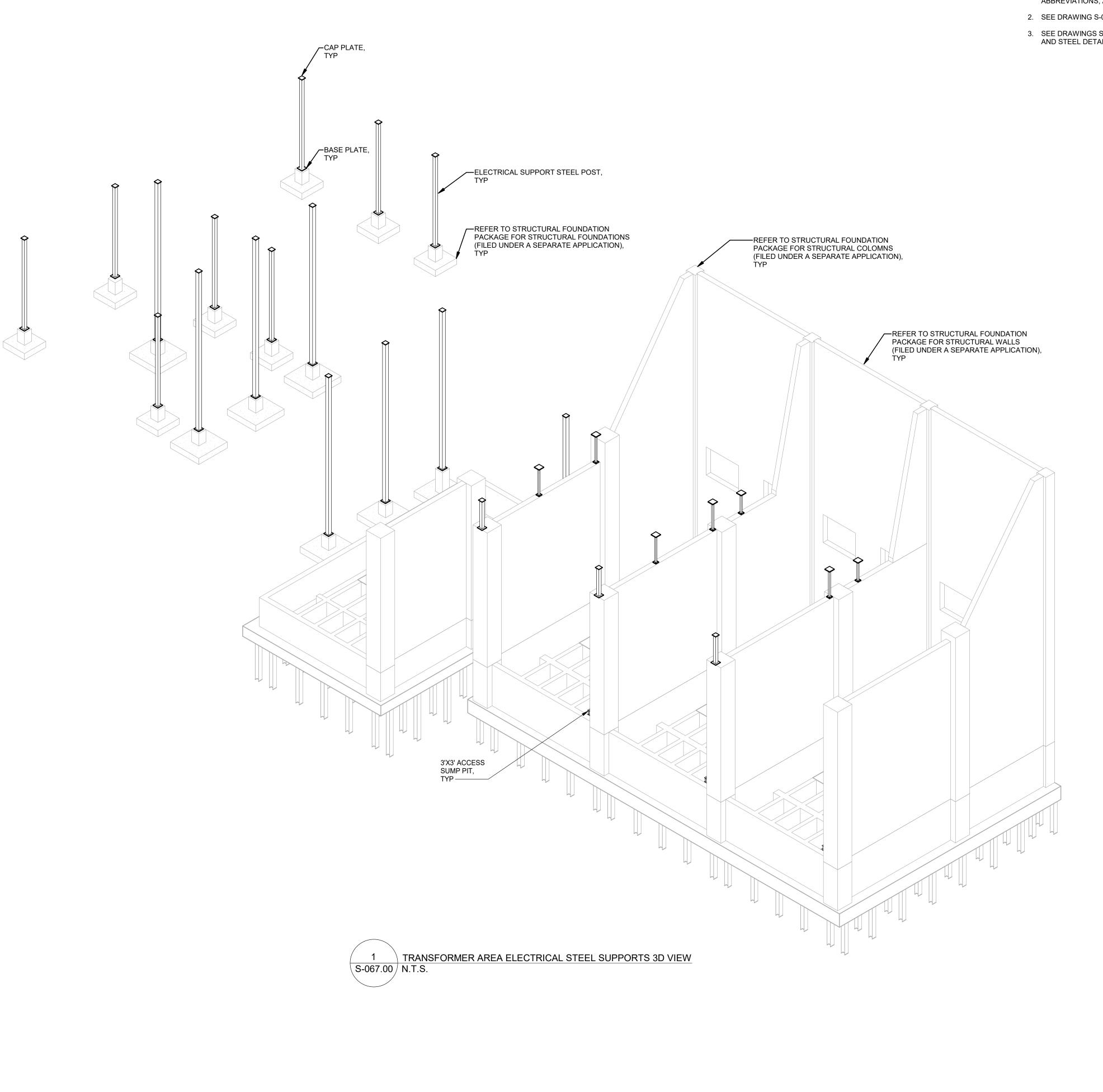
31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

AC FILTER ELECTRICAL STEEL SUPPORTS 3D VIEW

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
	000

S-066.00

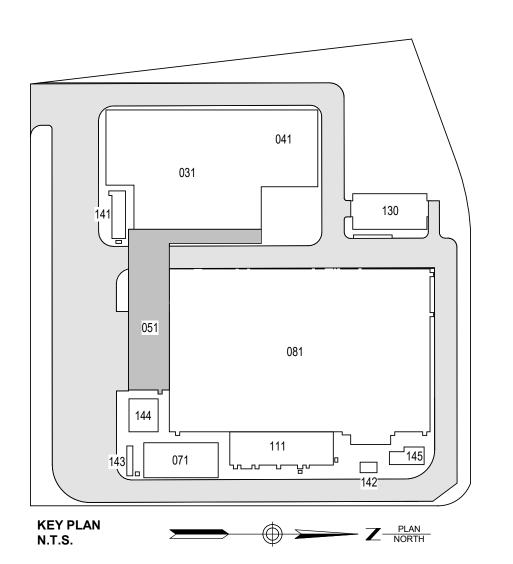




STRUCTURE NOTES:

- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. SEE DRAWING S-010.00 FOR STEEL LOCATION PLAN.
- 3. SEE DRAWINGS S-601.00 THRU S-617.00 FOR TYPICAL STEEL CONNECTIONS, AND STEEL DETAILS.

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В	FINAL SUBMISSION	DJF	AA	12/12/2022
Α	INTERIM SUBMISSION	GDB	DS	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



901 Main Campus Drive Raleigh, North Carolina 27606

PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

TRANSFORMER AREA ELECTRICAL STEEL SUPPORTS 3D VIEW

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
c	C700

S-U6 / .U0CADD FILE NO 14 of 49

Autodesk Docs://CHPE Astoria/CHA-KIE-051-ZZ-M3-S-001.rvt

TRANSFORMER AREA ELECTRICAL SUPPORTS SHEET INDEX

SHEET NUMBER

TRANSFORMER AREA ELECTRICAL STEEL SUPPORTS 3D VIEW

TRANSFORMER AREA ELECTRICAL SUPPORT SECTIONS AND DETAILS

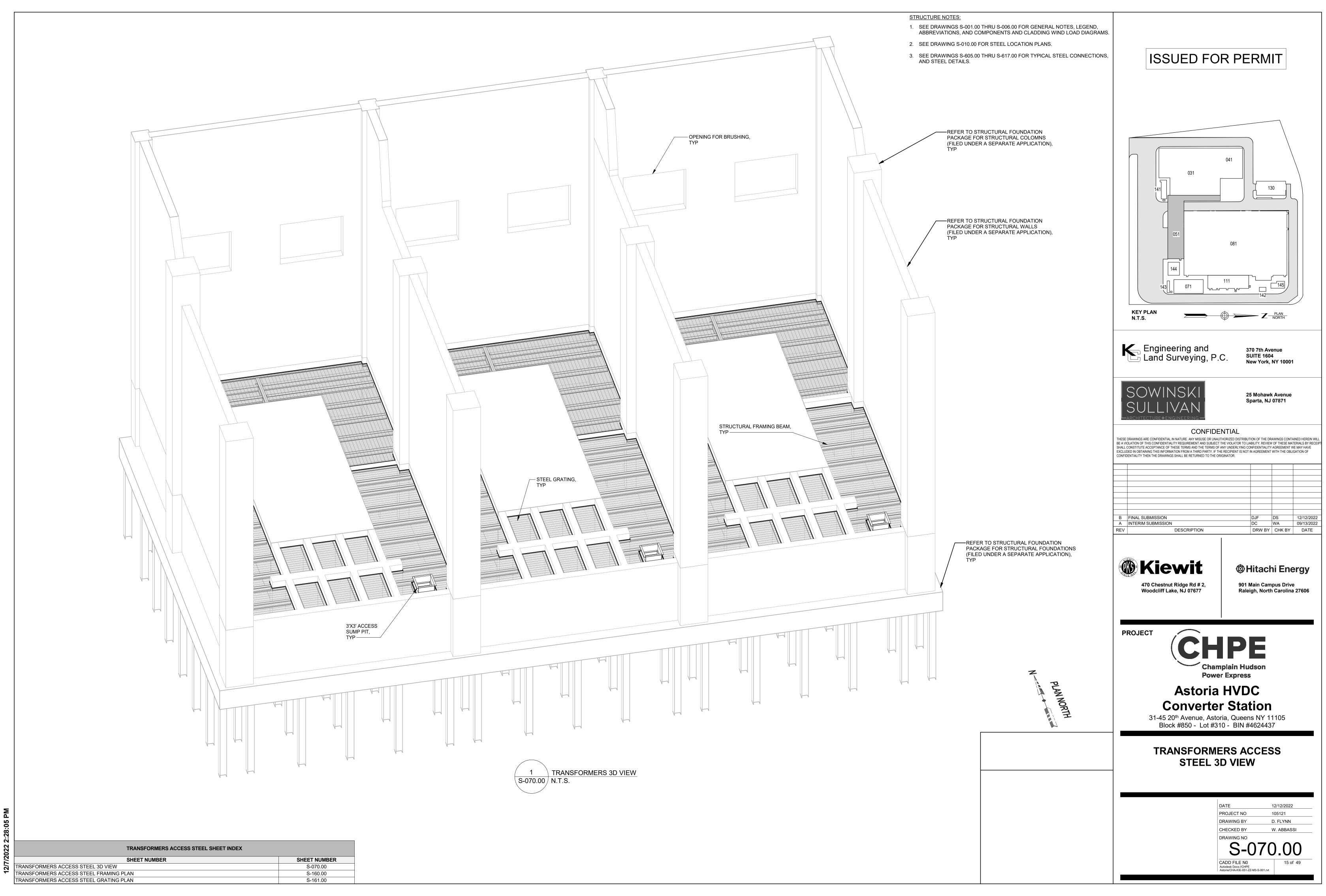
TRANSFORMER AREA A ELECTRICAL STEEL SUPPORTS LOCATION PLAN AND TABLE TRANSFORMER AREA B ELECTRICAL STEEL SUPPORTS LOCATION PLAN AND TABLE

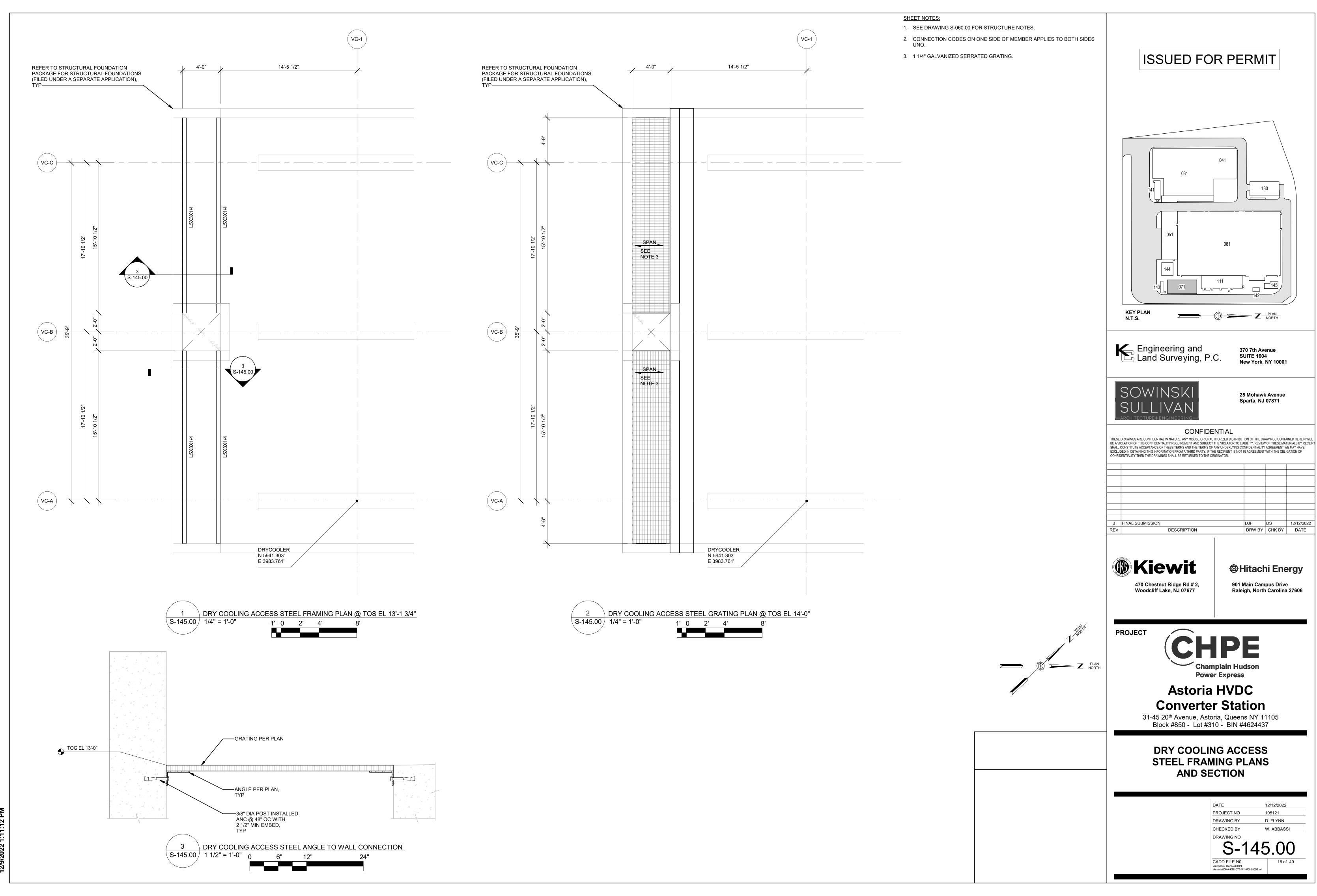
SHEET NUMBER

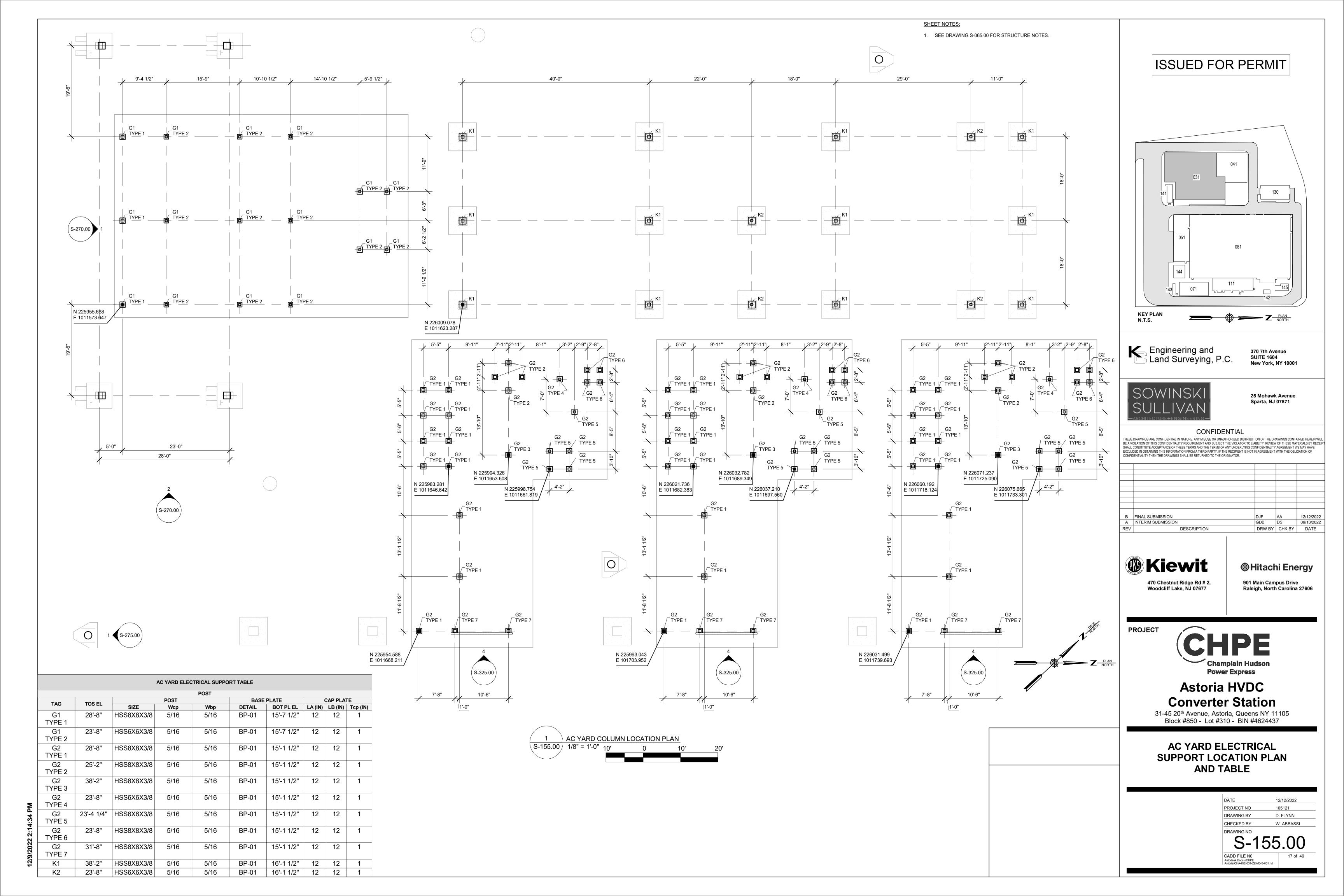
S-067.00 S-157.00

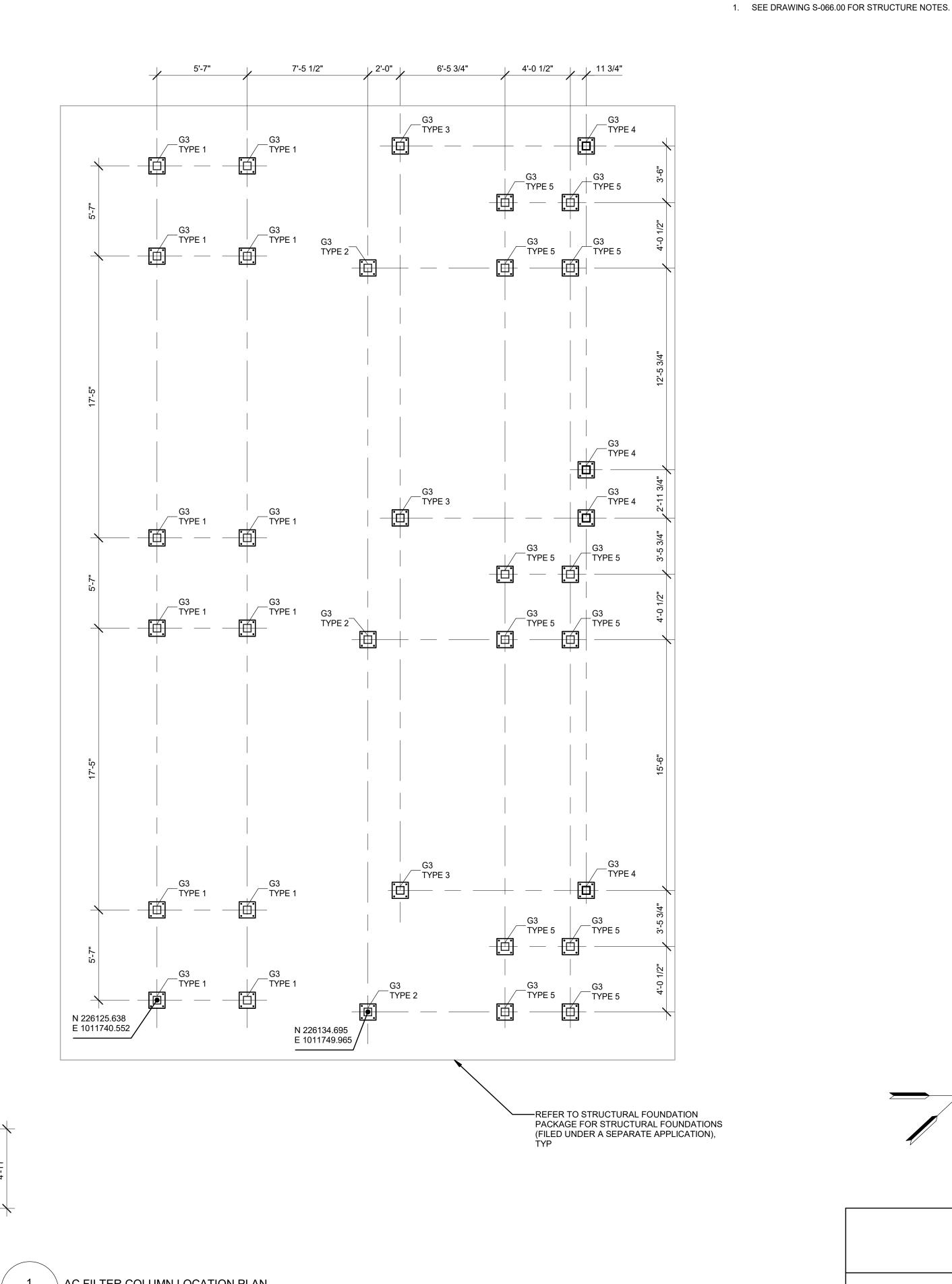
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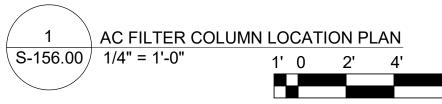
S-327.00



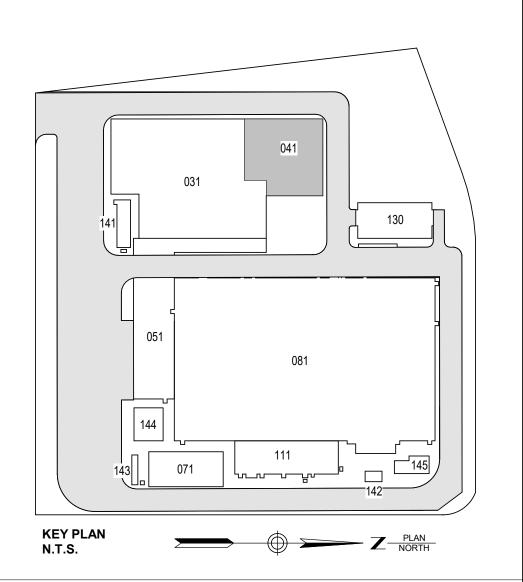








SHEET NOTES:



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В	FINAL SUBMISSION	DJF	AA	12/12/2022
Α	INTERIM SUBMISSION	GDB	DS	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE
•				



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Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

AC FILTER ELECTRICAL SUPPORT LOCATION PLAN **AND TABLE**

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
A	

S-156.00 CADD FILE NO
Autodesk Docs://CHPE
Astoria/CHA-KIE-041-ZZ-M3-S-001.rvt

TAG

G3

TYPE 1

G3

TYPE 2

G3

TYPE 3

G3

TYPE 4

G3

TYPE 5

28'-3 1/2" | HSS8X8X3/8 |

17'-1 1/8" | HSS6X6X3/8

HSS6X6X3/8

5/16

18'-5 1/4" HSS6X6X3/8 5/16

20'-1" HSS6X6X3/8 5/16

HSS6X6X3/8

S-326.00

CAP PLATE

12 | 12 | 1

12 | 12 |

12 12

BOT PL EL LA (IN) LB (IN) Tcp (IN) 1<u>2</u> 1

15'-7 1/2" | 12 | 12

15'-7 1/2" | 12 | 12 | 1

N 226105.672 E 1011739.571

C1—

N 226106.862 E 1011712.464

BASE PLATE

BP-01

15'-7 1/2" 15'-7 1/2"

15'-5 1/4"

<varies>

AC FILTER AREA ELECTRICAL SUPPORT TABLE

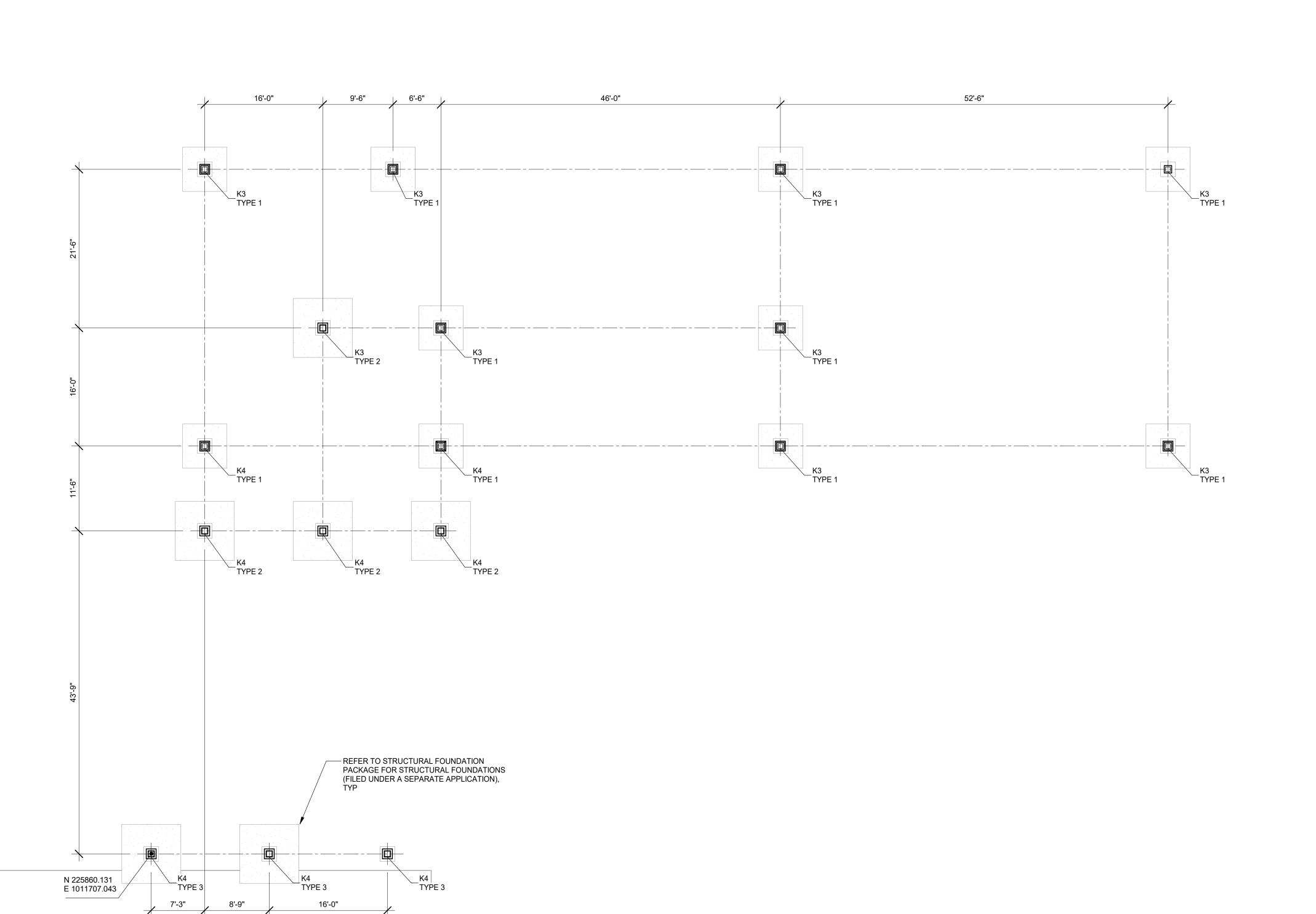
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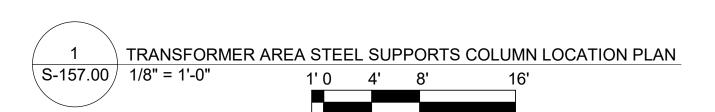
5/16

5/16

5/16

5/16

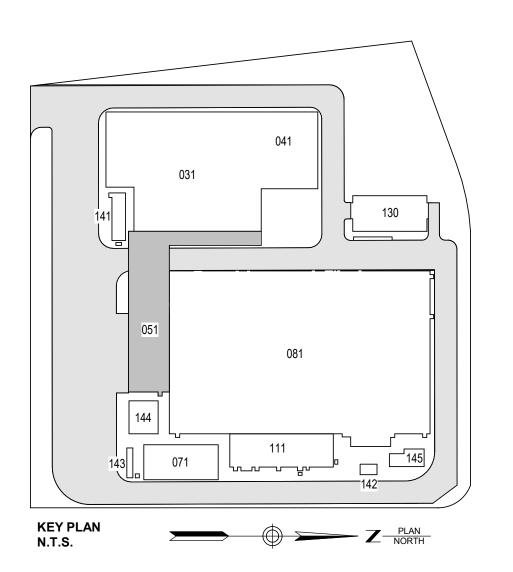




	TRANSFORMER AREA A ELECTRICAL SUPPORT TABLE										
	TAG	TOS EL	POST		BASE PLATE		CAP PLATE				
12/7/2022 2:28:07 PM	IAG	103 EL	SIZE	Wcp	Wbp	DETAIL	BOT PL EL	LA (IN)	LB (IN)	Tcp (IN)	
	K3 TYPE 1	38'-2"	HSS8X8X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1	
	K3 TYPE 2	54'-8"	HSS10X10X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1	
	K4 TYPE 1	38'-2"	HSS8X8X3/8	5/16	5/16	BP-01	16'-2"	12	12	1	
	K4 TYPE 2	54'-8"	HSS10X10X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1	
12	K4 TYPF 3	54'-8"	HSS10X10X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1	

SHEET NOTES:

1. SEE DRAWING S-067.00 FOR STRUCTURE NOTES.



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В	FINAL SUBMISSION	DJF	AA	12/12/2022
Α	INTERIM SUBMISSION	GDB	DS	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



901 Main Campus Drive Raleigh, North Carolina 27606



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

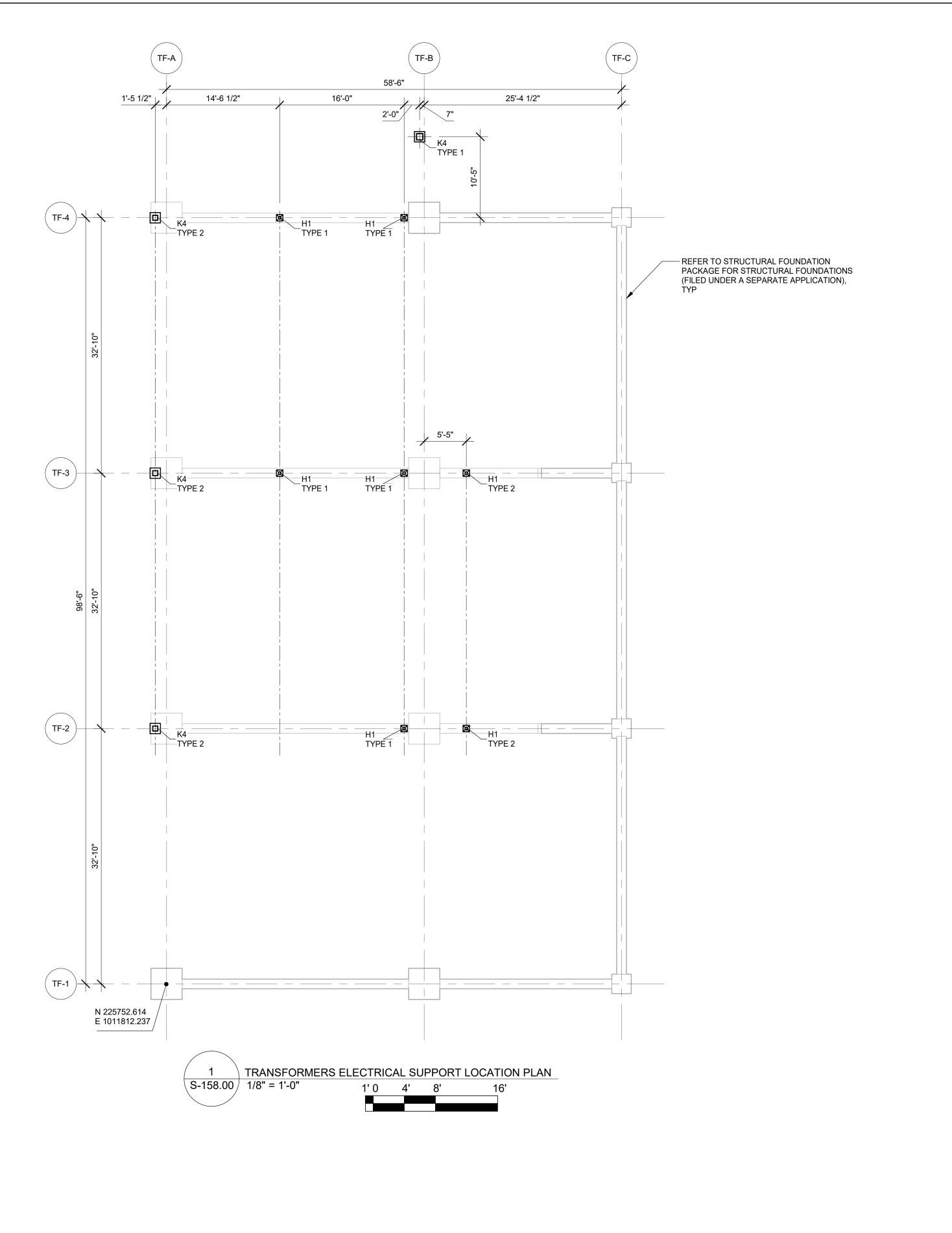
TRANSFORMER AREA A **ELECTRICAL STEEL SUPPORTS LOCATION PLAN AND TABLE**

DATE		•	12/12/2	022	
PROJECT NO			105121		
DRAWING BY		[D. FLY	NN	
CHECKED BY		١	W. ABE	BASSI	
DRAWING NO					
	4		- 4	^ ^	

S-157.00

CADD FILE NO
Autodesk Docs://CHPE
Astoria/CHA-KIE-051-ZZ-M3-S-001.rvt

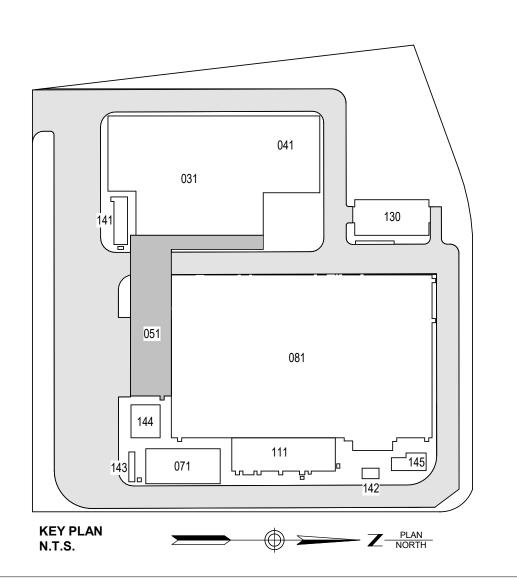
	TRANSFORMER AREA A ELECTRICAL SUPPORT TABLE									
	TAG	TOS EL		POST		BASE	PLATE	CAP PLATE		
	IAG	103 EL	SIZE	Wcp	Wbp	DETAIL	BOT PL EL	LA (IN)	LB (IN)	Tcp (IN)
ΡM	K3 TYPE 1	38'-2"	HSS8X8X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1
2:28:07	K3 TYPE 2	54'-8"	HSS10X10X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1
	K4 TYPE 1	38'-2"	HSS8X8X3/8	5/16	5/16	BP-01	16'-2"	12	12	1
12/7/2022	K4 TYPE 2	54'-8"	HSS10X10X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1
12	K4 TYPE 3	54'-8"	HSS10X10X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1



SHEET NOTES:

- 1. SEE DRAWING S-067.00 FOR STRUCTURE NOTES.
- 2. CONNECTION CODES ON ONE SIDE OF MEMBER APPLIES TO BOTH SIDES UNLESS NOTED OTHERWISE.

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I	В	FINAL SUBMISSION	DJF	AA	12/12/2022
	Α	INTERIM SUBMISSION	DC	WA	09/13/2022
	REV	DESCRIPTION	DRW BY	CHK BY	DATE
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ROJECT



Astoria HVDC Converter Station

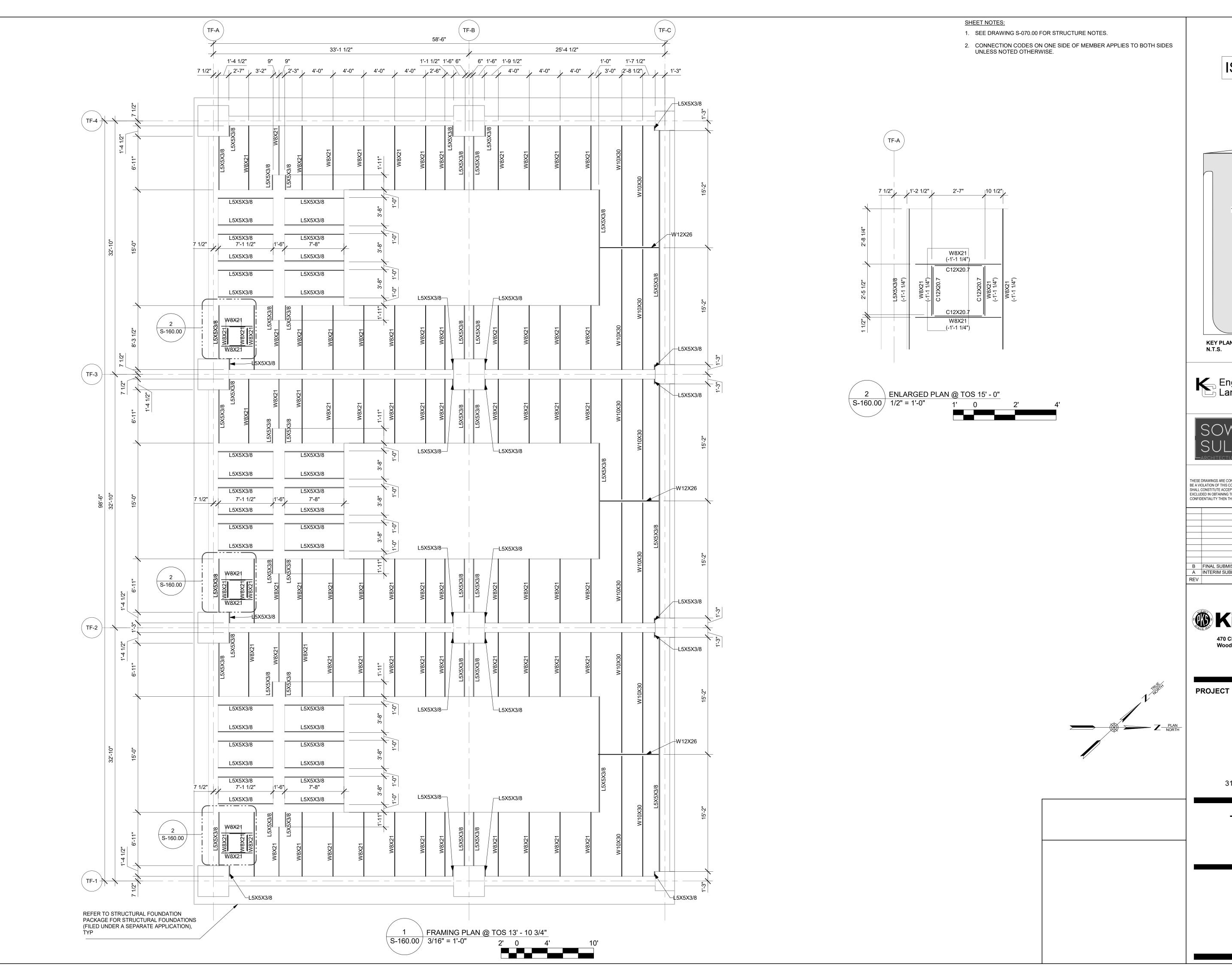
31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

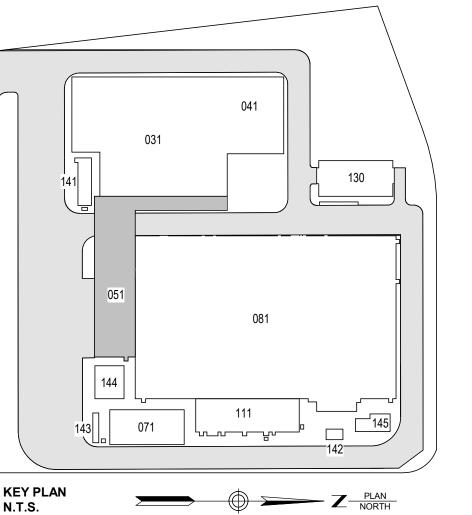
TRANSFORMER AREA B
ELECTRICAL STEEL
SUPPORTS LOCATION
PLAN AND TABLE

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
\mathbf{C} 1	50 NN

CADD FILE N0
Autodesk Docs://CHPE
Astoria/CHA-KIE-051-ZZ-M3-S-001.rvt

		TRANSFORMER AREA B ELECTRICAL SUPPORT TABLE								
12/7/2022 2:28:08 PM	TAC	TOC 51		POST		BASE	PLATE		CAP PLA	ΓΕ
	TAG	TOS EL	SIZE	Wcp	Wbp	DETAIL	BOT PL EL	LA (IN)	LB (IN)	Tcp (IN)
	H1 TYPE 1	54'-6 1/2"	HSS4X4X3/8	5/16	5/16	BP-01	47'-9"	12	12	1
	H1 TYPE 2	54'-8 1/2"	HSS4X4X3/8	5/16	5/16	BP-01	47'-9"	12	12	1
	K4 TYPE 1	52'-10 3/4"	HSS10X10X3/8	5/16	5/16	BP-01	16'-1 1/2"	12	12	1
12	K4 TYPF 2	54'-6 1/2"	HSS8X8X3/8	5/16	5/16	BP-01	47'-9"	12	12	1





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В	FINAL SUBMISSION	DJF	DS	12/12/2022
Α	INTERIM SUBMISSION	DC	WA	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE

Kiewit 470 Chestnut Ridge Rd # 2, Woodcliff Lake, NJ 07677

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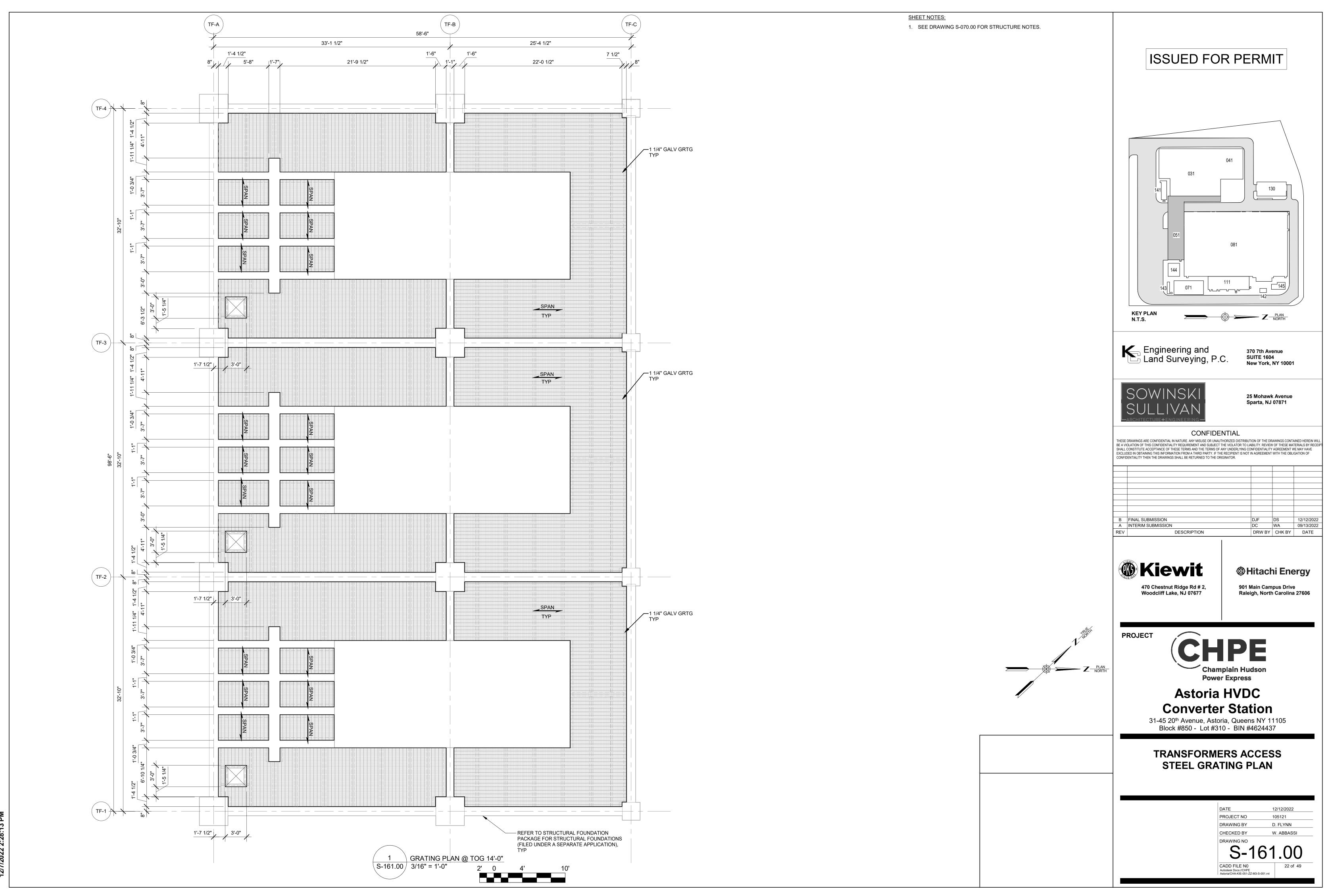
Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

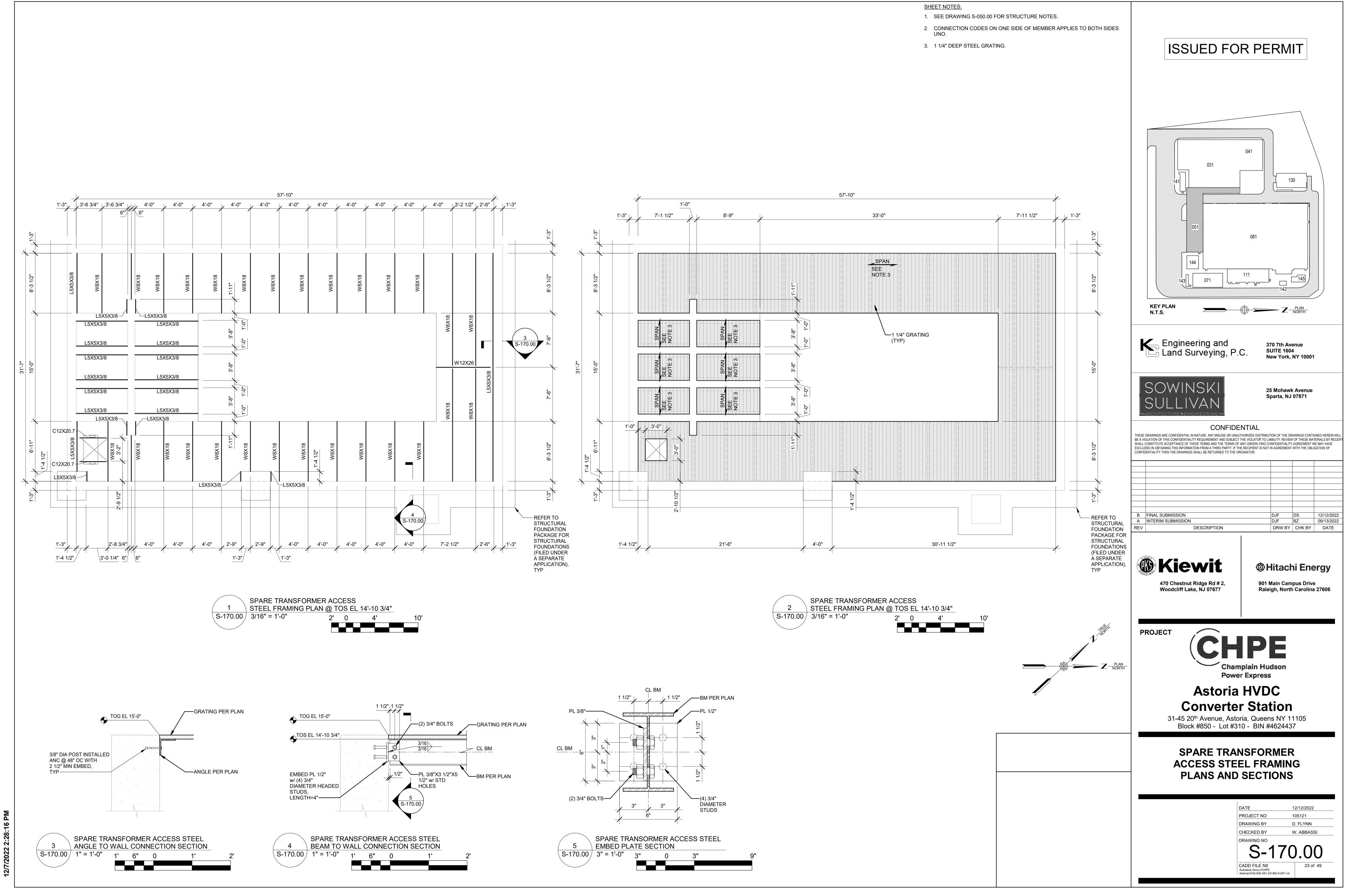
TRANSFORMERS ACCESS STEEL FRAMING PLAN

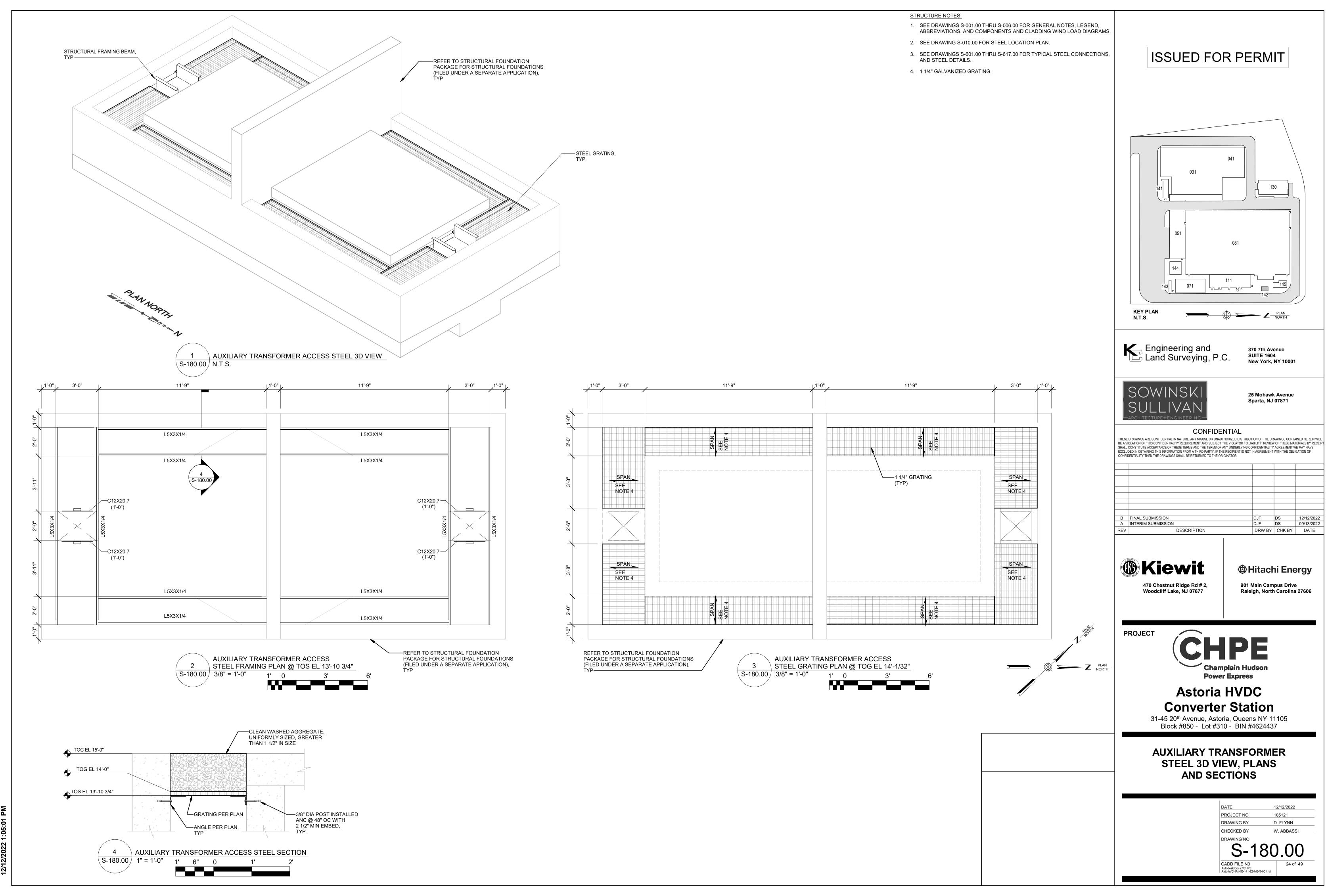
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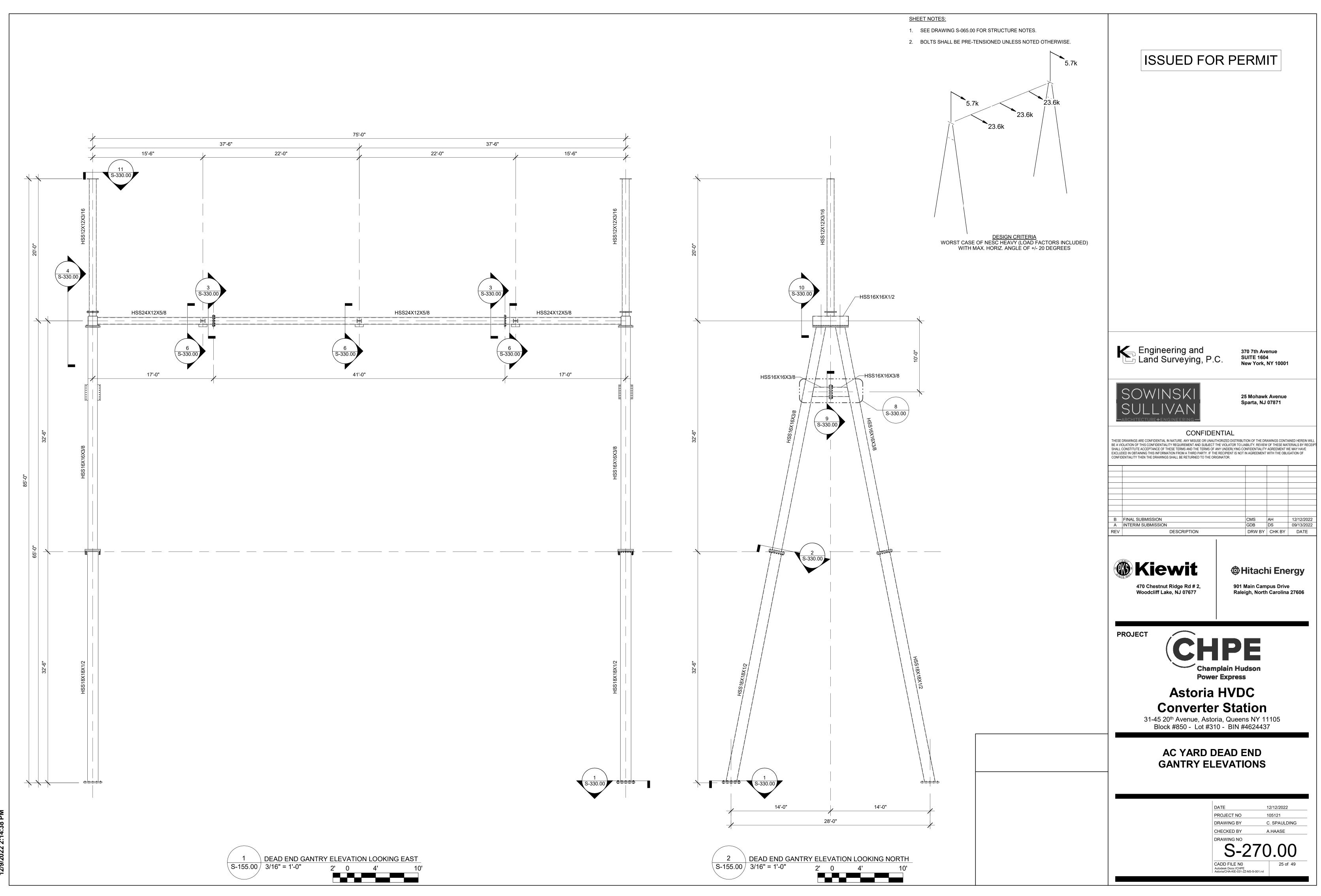
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Autodesk Docs://CHPE
Astoria/CHA-KIE-051-ZZ-M3-S-001.rvt



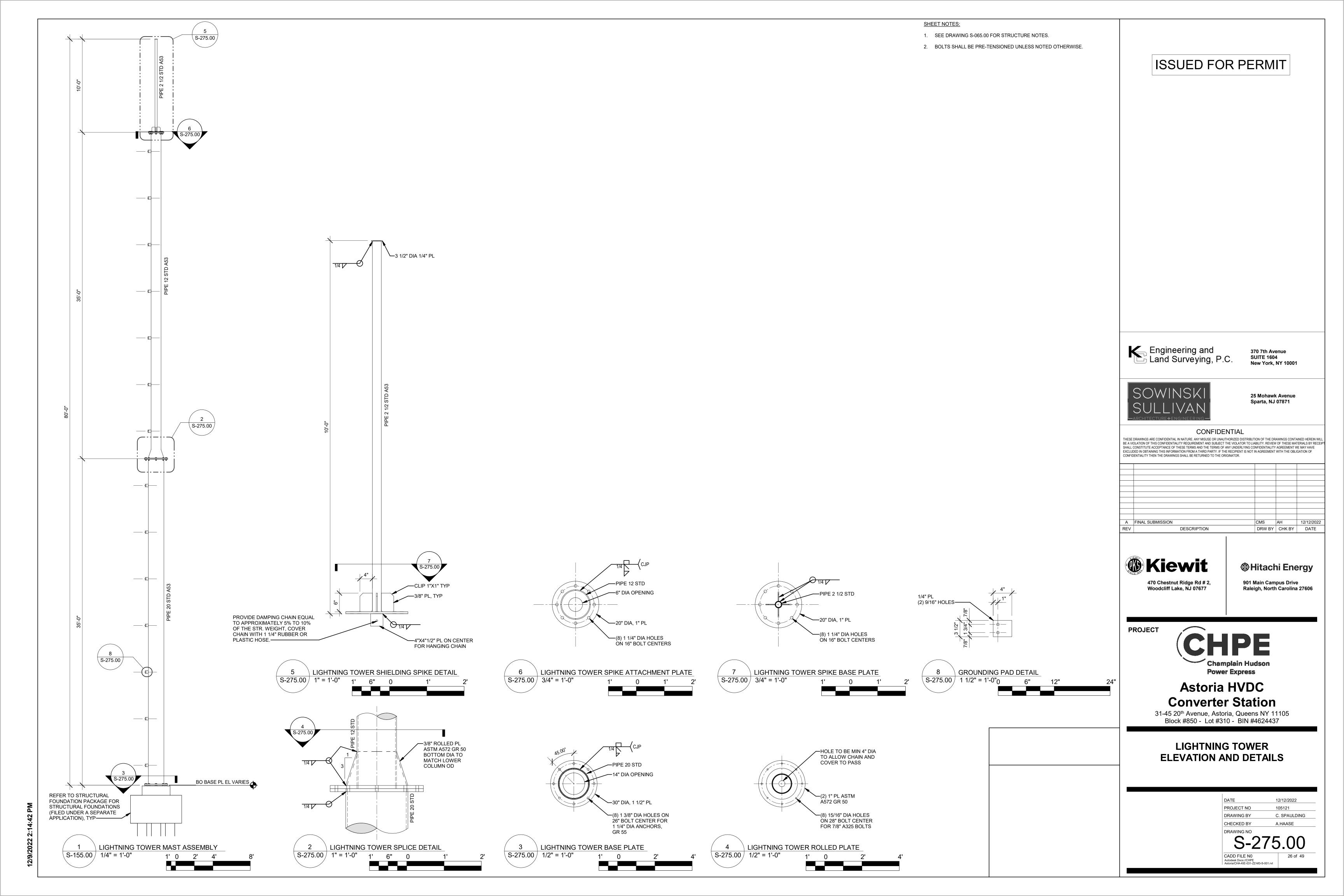
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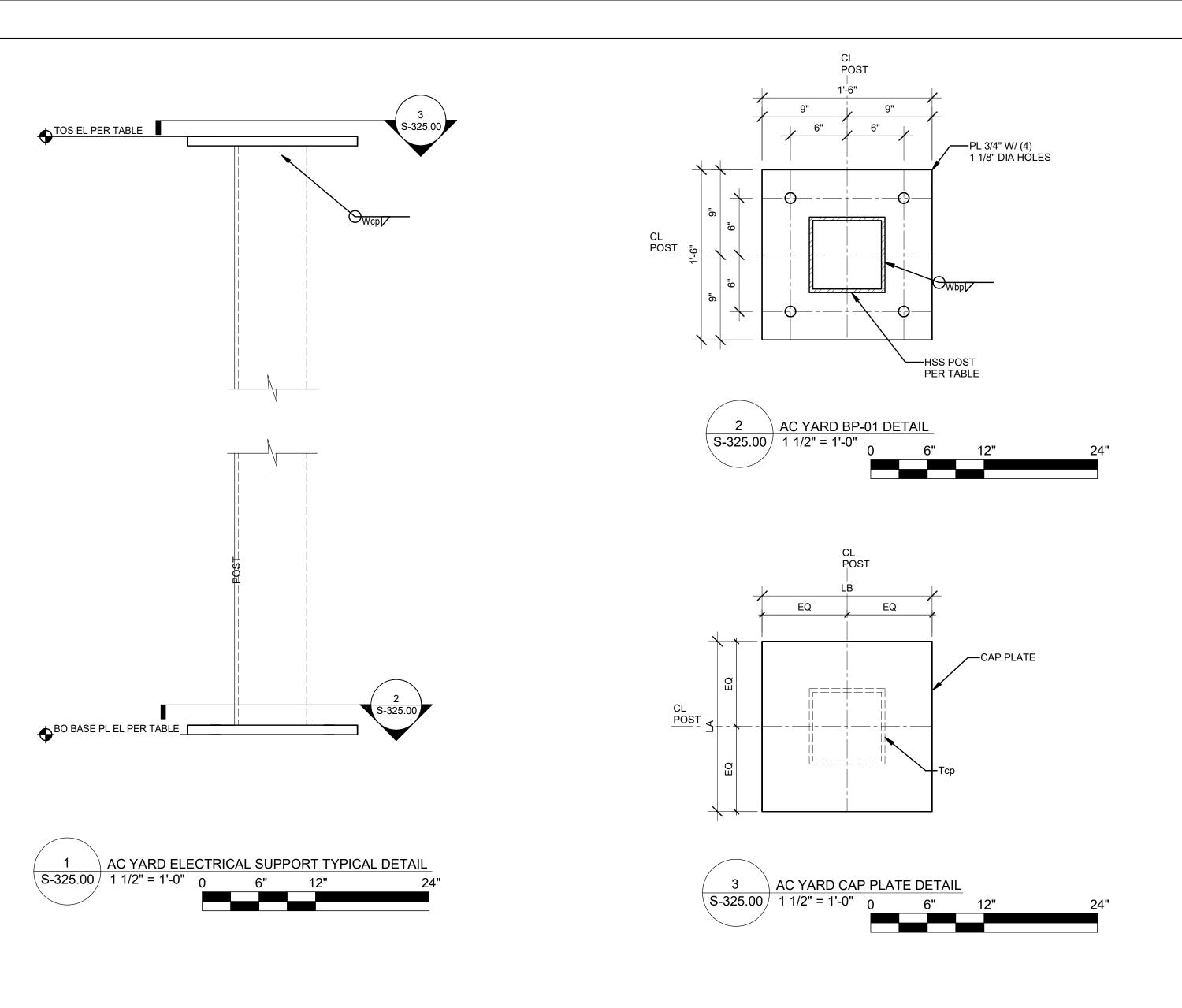


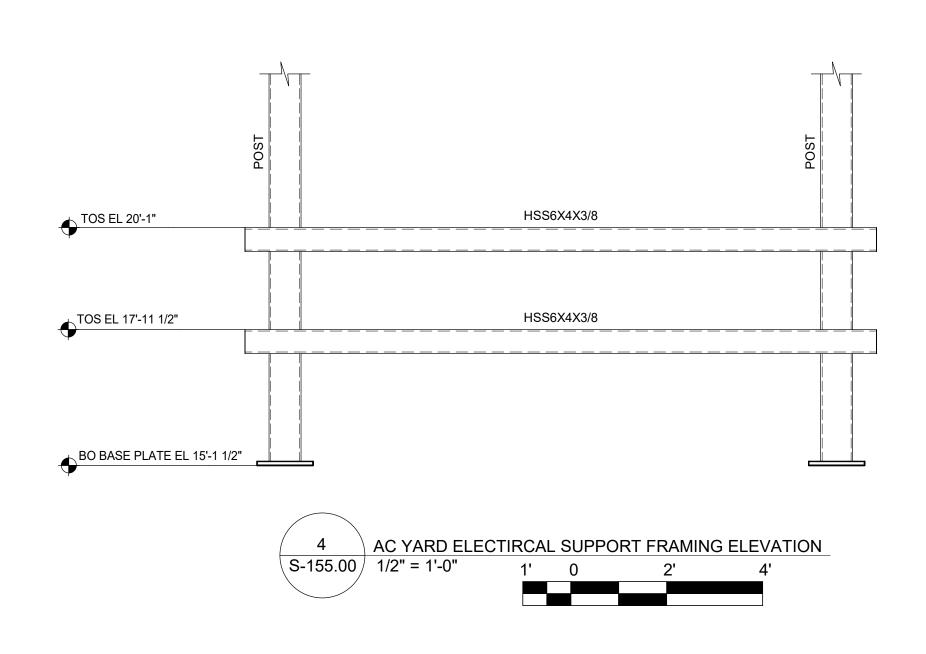




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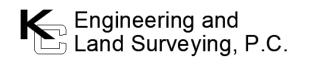




SHEET NOTES:

1. SEE DRAWING S-065.00 FOR STRUCTURE NOTES.

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В	FINAL SUBMISSION	DJF	AA	12/12/2022
Α	INTERIM SUBMISSION	GDB	DS	09/13/2022
RF\/	DESCRIPTION	DRW BY	CHK BY	DATE



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PROJECT

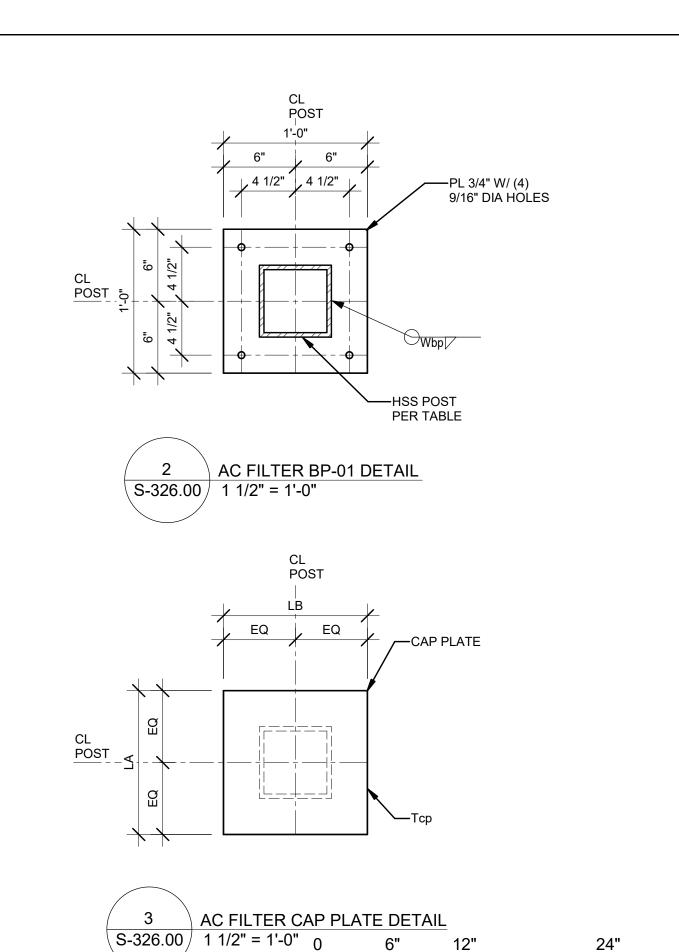


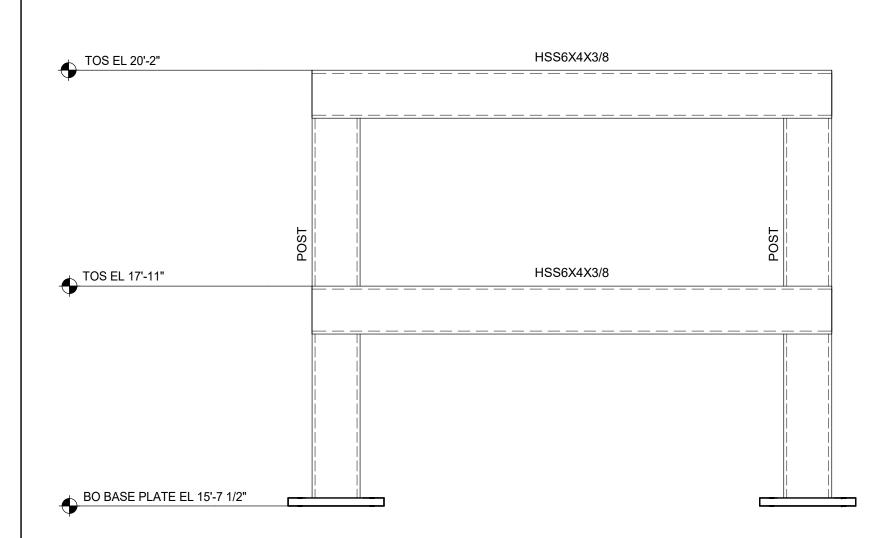
Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

AC YARD ELECTRICAL SUPPORT SECTIONS AND **DETAILS**

> PROJECT NO D. FLYNN CHECKED BY W. ABBASSI DRAWING NO

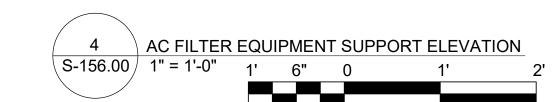




TOS EL PER TABLE

BO BASE PLATE EL 15'-7 1/2"

S-326.00 1 1/2" = 1'-0" 0

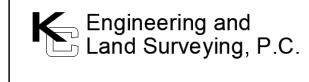


AC FILTER ELECTRICAL SUPPORT TYPICAL DETAIL

SHEET NOTES:

1. SEE DRAWING S-066.00 FOR STRUCTURE NOTES.

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В	FINAL SUBMISSION	DJF	AA	12/12/2022
Α	INTERIM SUBMISSION	GDB	DS	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



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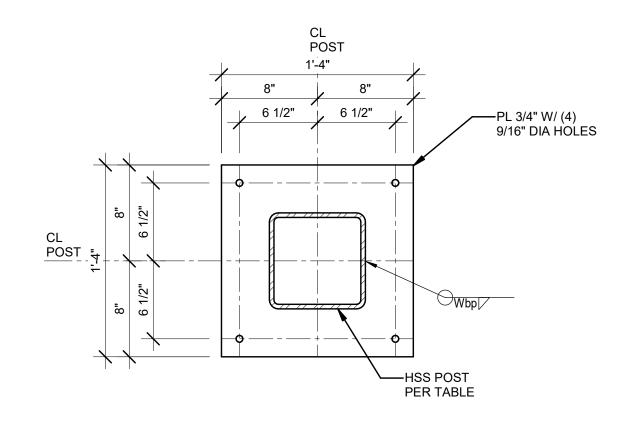
Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

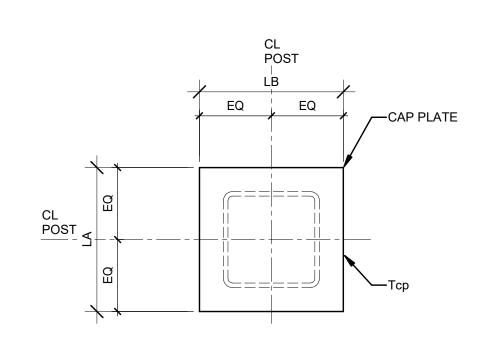
AC FILTER ELECTRICAL SUPPORT SECTIONS AND DETAILS

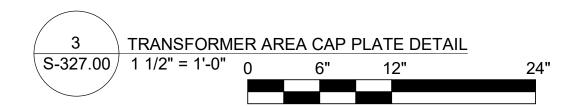
1	
DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DD AMUNICALO	

1. SEE DRAWING S-067.00 FOR STRUCTURE NOTES.











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B FINAL SUBMISSION					
A INTERIM SUBMISSION GDB DS 09/13/2022					
A INTERIM SUBMISSION GDB DS 09/13/2022					
A INTERIM SUBMISSION GDB DS 09/13/2022					
A INTERIM SUBMISSION GDB DS 09/13/2022					
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A INTERIM SUBMISSION GDB DS 09/13/2022					
A INTERIM SUBMISSION GDB DS 09/13/2022					
	В	FINAL SUBMISSION	DJF	AA	12/12/2022
REV DESCRIPTION DRW BY CHK BY DATE	Α	INTERIM SUBMISSION	GDB	DS	09/13/2022
	RE	DESCRIPTION	DRW BY	CHK BY	DATE



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PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

TRANSFORMER AREA ELECTRICAL SUPPORT SECTIONS AND DETAILS

 DATE
 12/12/2022

 PROJECT NO
 105121

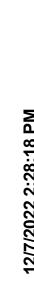
 DRAWING BY
 D. FLYNN

 CHECKED BY
 W. ABBASSI

 DRAWING NO

S-327.00

CADD FILE NO
Autodesk Docs://CHPE
Astoria/CHA-KIE-051-ZZ-M3-S-001.rvt

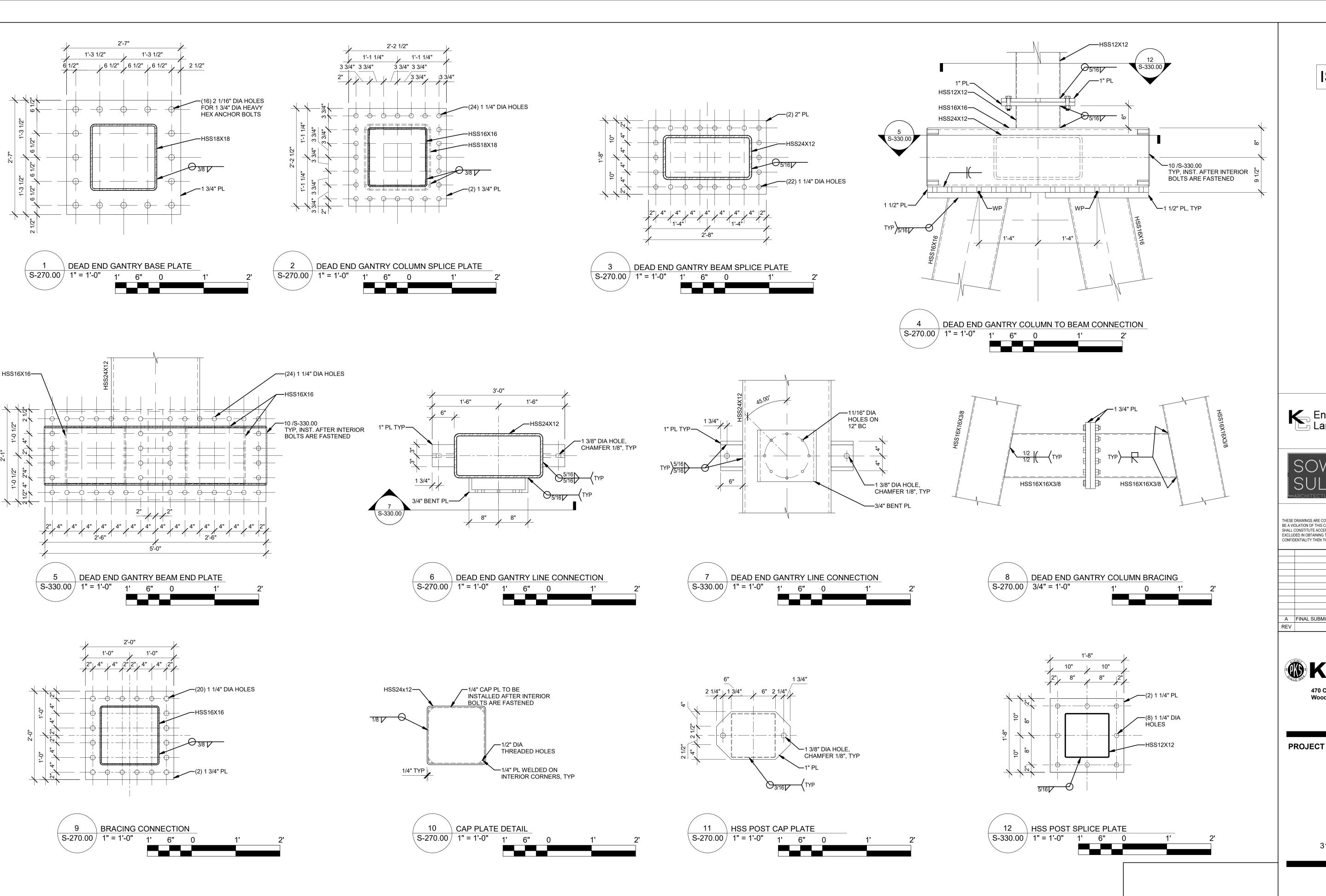


TOS EL PER TABLE

BO BASE PLATE EL 16'-1 1/2"

1 TRANSFORMER AREA ELECTRICAL SUPPORT TYPICAL DETAIL

S-327.00 1 1/2" = 1'-0" 0 6" 12"



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A FINAL SUBMISSION 12/12/2022 DESCRIPTION DRW BY CHK BY DATE



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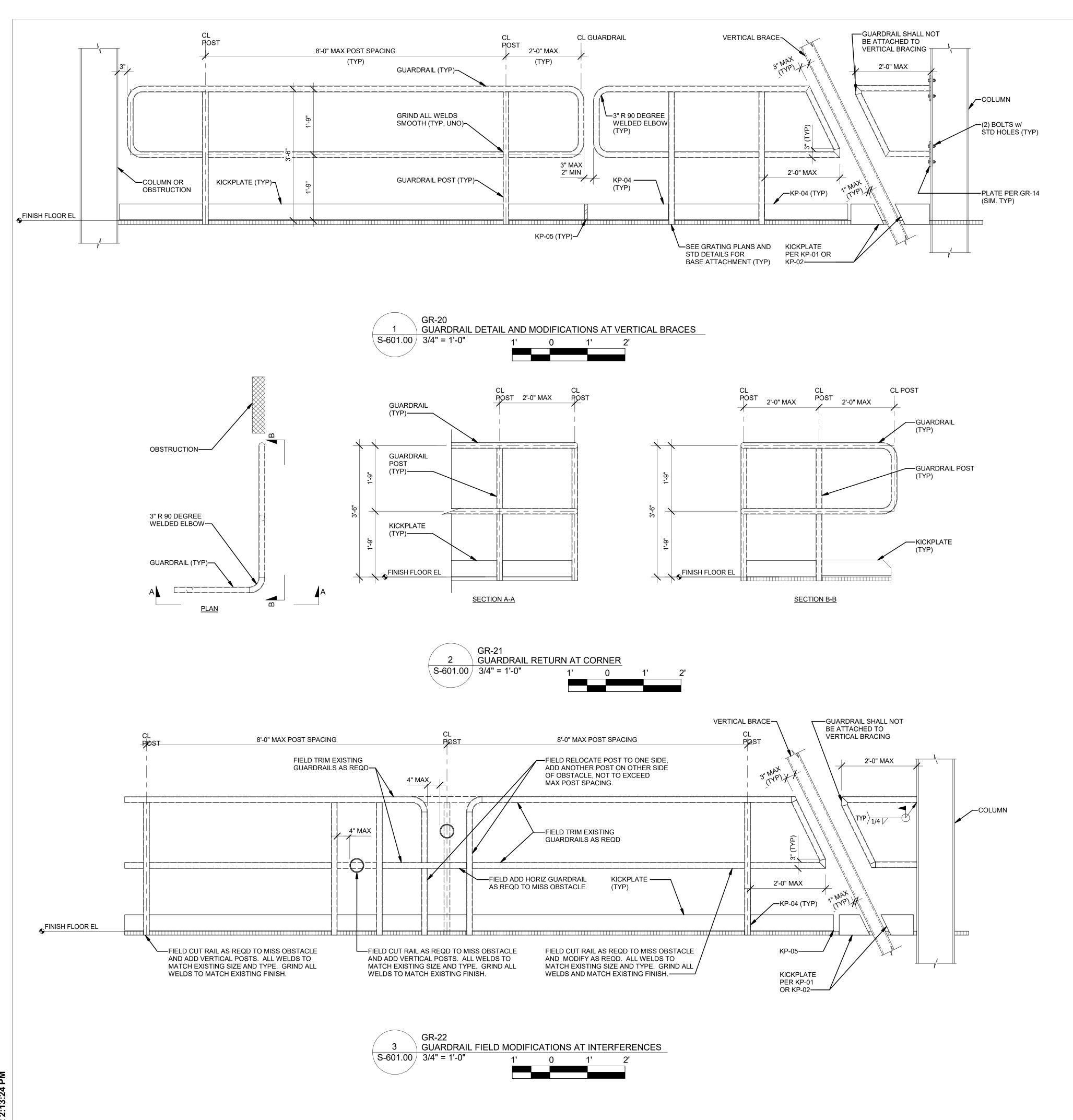
Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

AC YARD DEAD END GANTRY SECTIONS AND DETAILS

> 12/12/2022 PROJECT NO 105121 DRAWING BY C. SPAULDING CHECKED BY A.HAASE DRAWING NO

CADD FILE NO
Autodesk Docs://CHPE
Astoria/CHA-KIE-031-ZZ-M3-S-001.rvt



SHEET NOTES:

- SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. 7/8" DIAMETER A325N BOLTS WITH 1 1/2" MINIMUM EDGE DISTANCE WITH STANDARD HOLES @ 3" BOLT SPACING, UNO.
- 3. CONNECTIONS ARE SCHEMATICALLY SHOWN FOR DESIGN INTENT. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR ERECTOR. BOLTS SHOWN AS HEX BOLTS MAY BE TC BOLTS AS DETERMINED BY THE GENERAL NOTES

ISSUED FOR PERMIT



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25 Mohawk Avenue



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В	FINAL SUBMISSION	DJF	WA	12/12/2022	
Α	INTERIM SUBMISSION	DJF	WA	09/13/2022	
REV	DESCRIPTION	DRW BY	CHK BY	DATE	



Hitachi Energy901 Main Campus DriveRaleigh, North Carolina 27606

PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

GUARDRAIL TYPICAL DETAILS

 DATE
 12/12/2022

 PROJECT NO
 105121

 DRAWING BY
 D. FLYNN

 CHECKED BY
 W. ABBASSI

 DRAWING NO

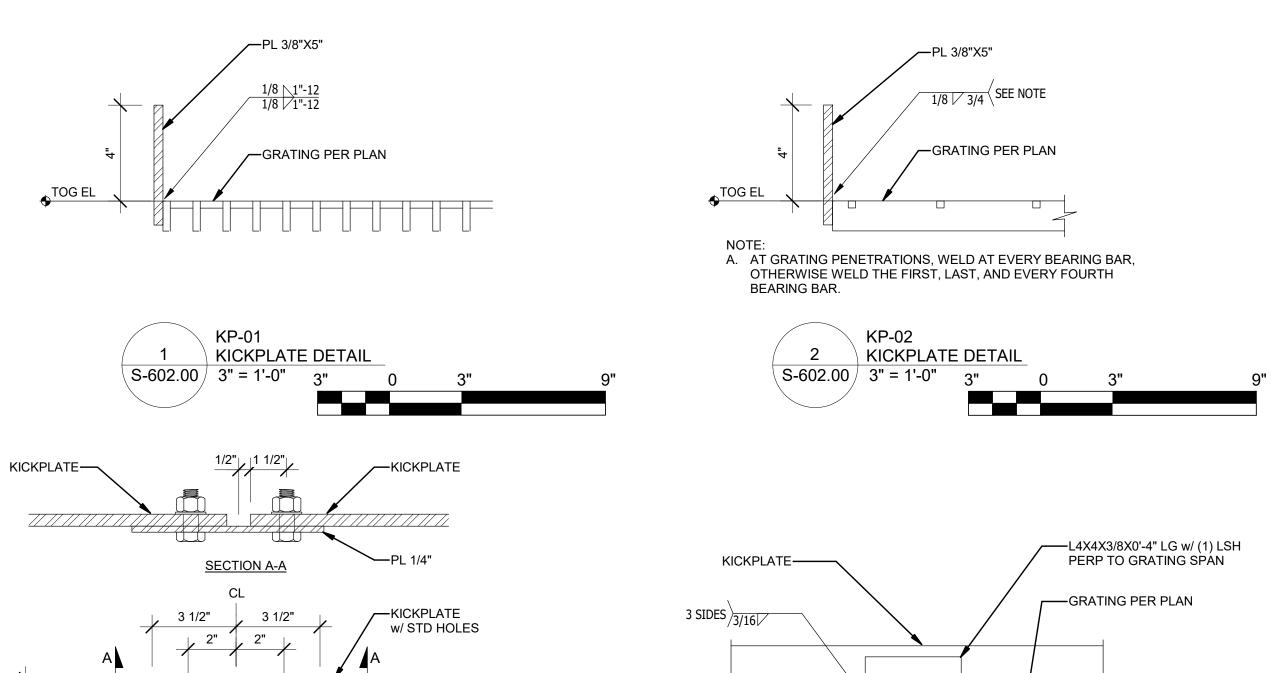
S-601.00

CADD FILE N0 Autodesk Docs://CHPE Astoria/CHA-KIE-000-XX-M2-S-001.rvt

SHEET NOTES:

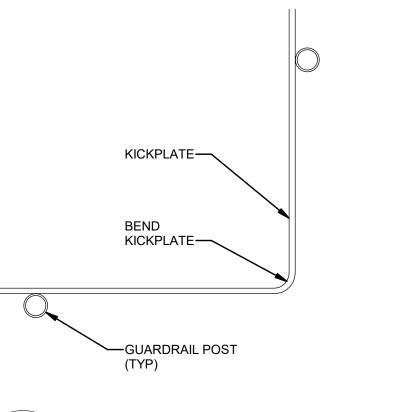
- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. AT GRATING PENETRATIONS, WELD AT EVERY BEARING BAR. OTHERWISE WELD THE FIRST, LAST, AND EVERY FOURTH BEARING BAR.

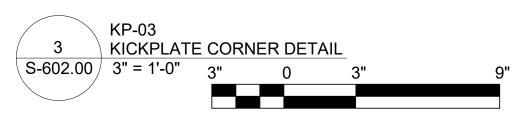
ISSUED FOR PERMIT

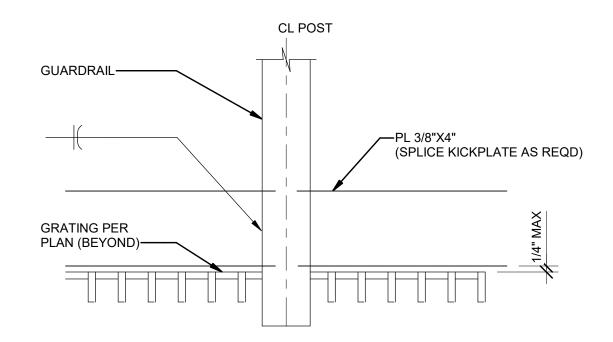


◆TOG EL

-PL 1/4" w/ HORIZONTAL SHORT SLOTTED HOLES











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REV	DESCRIPTION	DRW BY	CHK BY	DATE



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PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

KICKPLATE TYPICAL DETAILS

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
- C C(Ω

S-602.00

CADD FILE NO
Autodesk Docs://CHPE
Astoria/CHA-KIE-000-XX-M2-S-001.rvt

10 90.07.0 0.000/1/04

KICKPLATE

w/ STD HOLES-

NOTE:
ALTERNATIVELY, SPLICE PLATE CAN BE WELDED WITH 3/16 FILLET WELDS TO

1/2" SHELF DIMENSION TO ALLOW FOR THE FILLET WELD ON ALL SIDES.

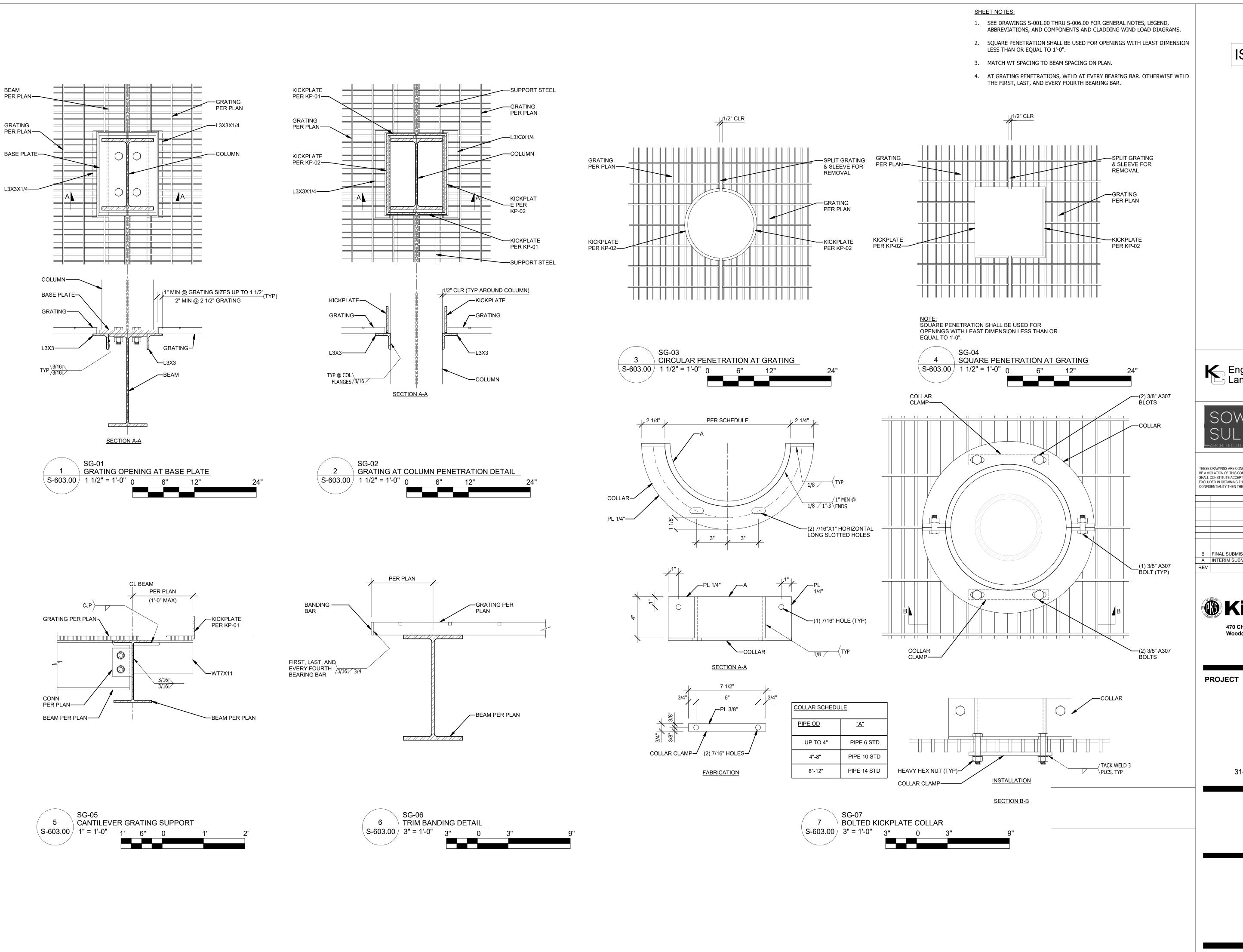
S-602.00 3" = 1'-0" 3"

BOTH KICKER PLATES. IN THE EVENT OF A WELDED CONNECTION, PROVIDE MIN.

KP-05 KICKPLATE SPLICE

6 KP-06 KICKPLATE CONNECTION S-602.00 3" = 1'-0" 3" 0 3"

SADDLE CLIP



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Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

GRATING TYPICAL DETAILS

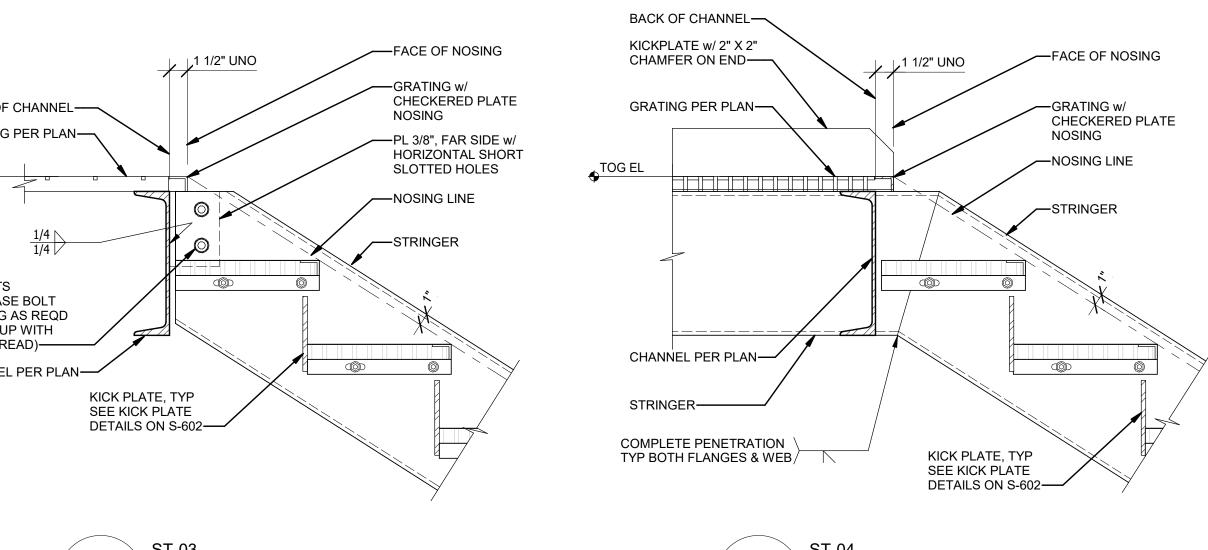
DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
_	

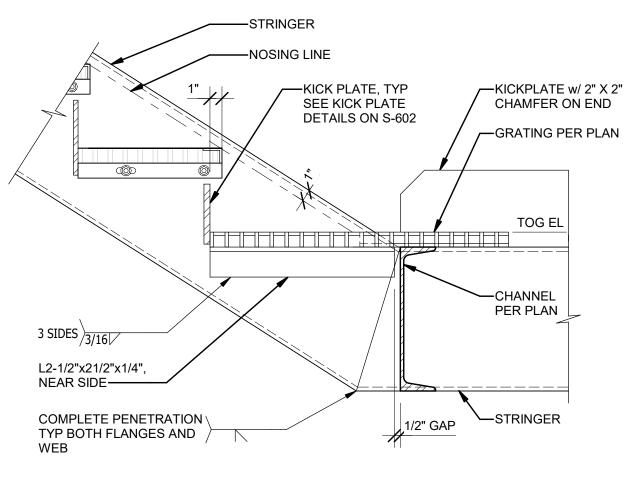
S-603.00

CADD FILE NO
Autodesk Docs://CHPE
Astoria/CHA-KIE-000-XX-M2-S-001.rvt



- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. CONTRACTOR TO REMOVE BOTTOM TREAD WHEN DRILLING FOR POST-INSTALLED ANCHOR.



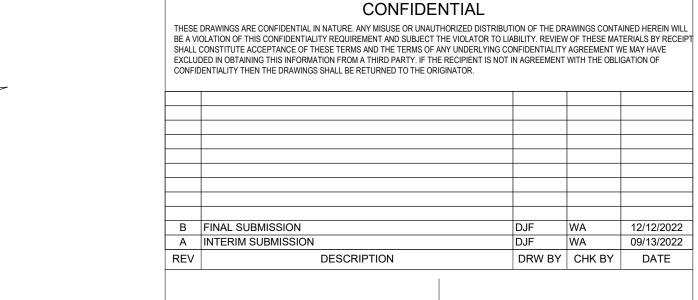


BENT STRINGER AT BOTTOM

S-604.00 / 1 1/2" = 1'-0" 0

BENT STRINGER AT TOP

S-604.00 / 11/2" = 1'-0" 0



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New York, NY 10001

25 Mohawk Avenue

Sparta, NJ 07871

SUITE 1604

PROJECT



Converter Station 31-45 20th Avenue, Astoria, Queens NY 11105

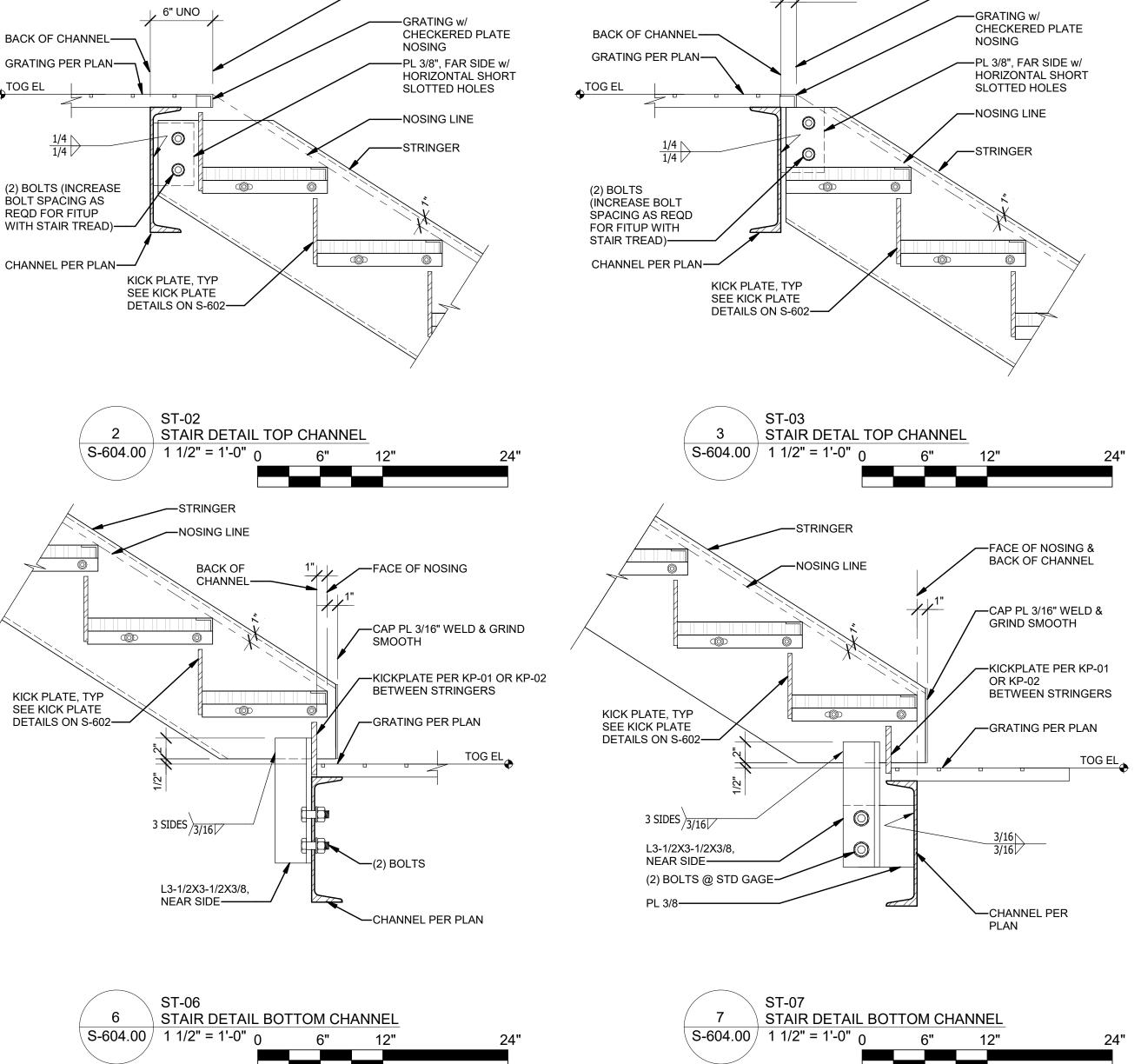
Block #850 - Lot #310 - BIN #4624437

STEEL STAIR TYPICAL CONNECTIONS

1	
DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
96	Ω
)\

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STAIR DETAIL BOTTOM SLAB $\langle S-604.00 \rangle 11/2" = 1'-0"_0$



—FACE OF NOSING

-KICK PLATE, TYP SEE KICK PLATE DETAILS ON S-602 5" FACE OF NOSING —CAP PL 3/16" WELD & **GRIND SMOOTH** ___L4X3X3/8X0'-4", LHH, **NEAR SIDE** 3 SIDES $\sqrt{\frac{3}{16}}$ -(1) 1/2" DIA X 0'-5 1/2" LG PÓST-INSTALLED ANCHORE w/ 3-1/4" MIN TOC EL CONCRETE FOUNDATION OR STAIR

STAIR DETAIL BOTTOM BEAM

✓ STRINGER

S-604.00 / 1 1/2" = 1'-0" 0

—FACE OF NOSING

PLATE NOSING

-PL 3/8", FAR SIDE w/

SLOTTED HOLES

-NOSING LINE

-STRINGER

KICK PLATE, TYP

SEE KICK PLATE

STAIR DETAIL TOP BEAM

-NOSING LINE

S-604.00 / 1 1/2" = 1'-0" 0

DETAILS ON S-602-

CL BEAM

—FACE OF NOSING

—CAP PL 3/16" WELD

& GRIND SMOOTH

KP-02 BETWEEN

—GRATING PER PLAN

STRINGERS

PLAN

-KICKPLATE PER KP-01 OR

TOG EL

HORIZONTAL SHORT

-GRATING w/ CHECKERED

GRATING

(2) BOLTS

(IŃCREASE BOLT

REQD FOR FITUP

BEAM PER PLAN

KICK PLATE, TYP

SEE KICK PLATE

3 SIDES > 3/16

L4X3X3/8X0'-3", LLV,

SIZES UP TO 1 1/2" L5X3X3/8X0'-3", LLV,

FAR SIDE @ 2 1/2"

GRATING—

(1) BOLT—

FAR SIDE @ GRATING

LANDING PAD-

DETAILS ON S-602-

SPACING AS

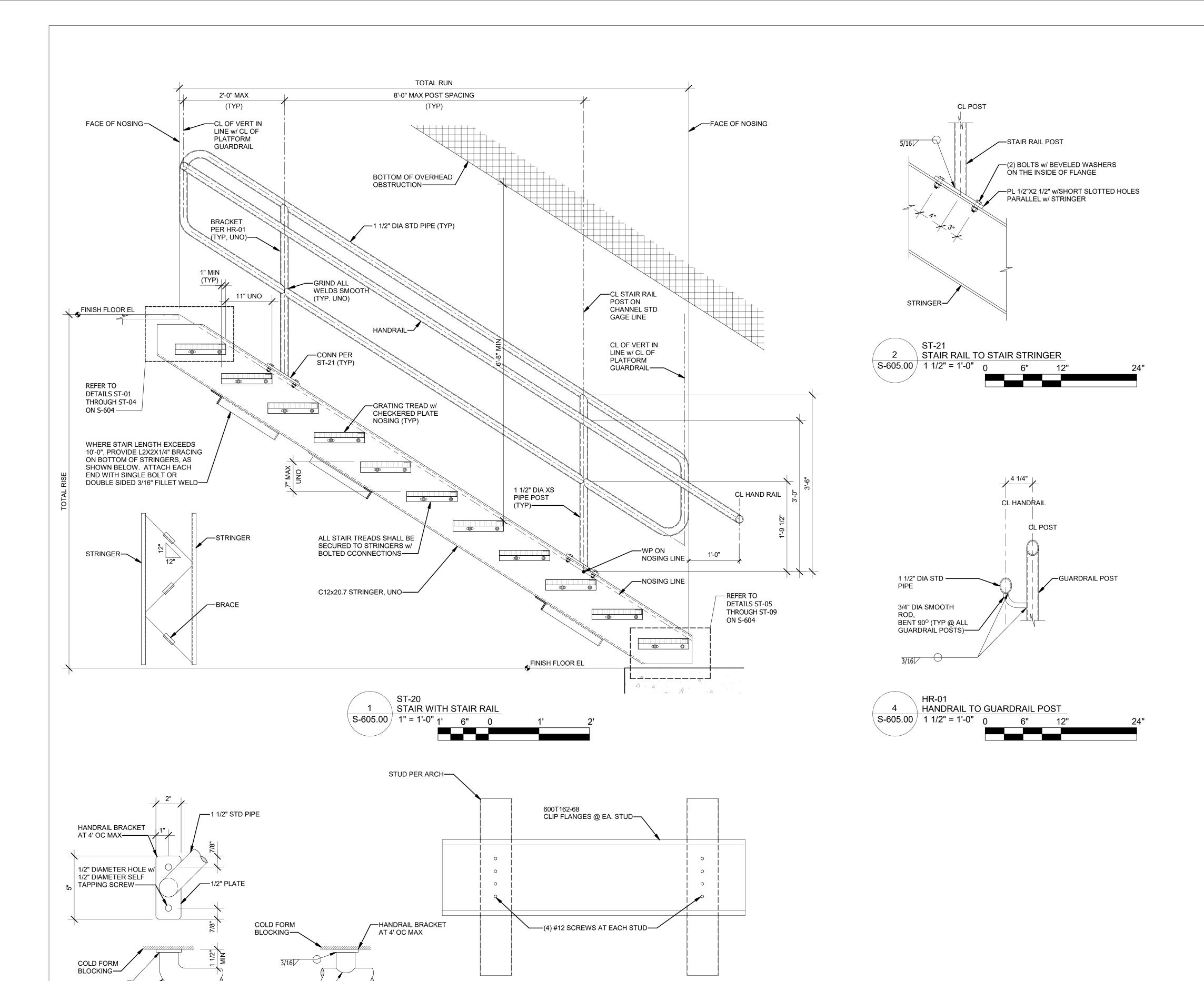
WITH STAIR

TREAD)—

PER PLAN-

BEAM 6" UNO

NOTE:
CONTRACTOR TO REMOVE BOTTOM THEAD WHEN DRILLING FOR POST INSTALLED ANCHOR.



COLD FORM BLOCKING

SHEET NOTES:

1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.

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PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

STEEL STAIR TYPICAL DETAILS

DATE 12/12/2022
PROJECT NO 105121
DRAWING BY D. FLYNN
CHECKED BY W. ABBASSI
DRAWING NO

S-605.00

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3/16

90° ELBOW —

3/16

HR-02 HANDRAIL TO WALL

S-605.00 3" = 1'-0" 3"



1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.

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[Α	INTERIM SUBMISSION	DJF	WA	09/13/2022
	REV	DESCRIPTION	DRW BY	CHK BY	DATE



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PROJEC



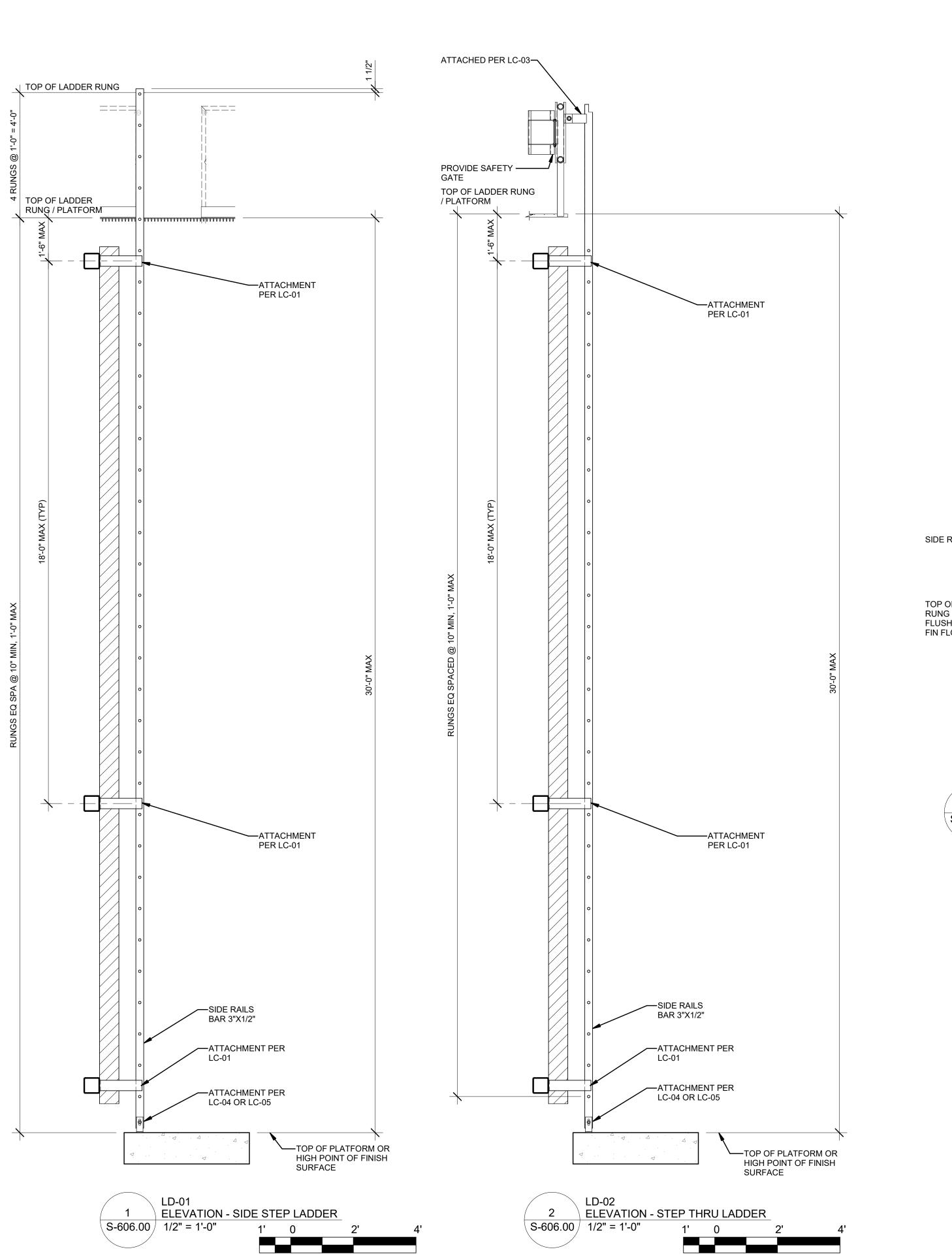
Astoria HVDC Converter Station

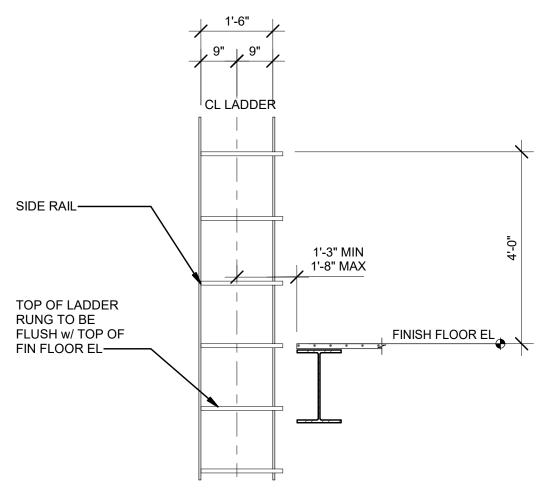
31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

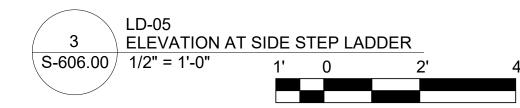
LADDER TYPICAL CONNECTIONS

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
CCO	$c \circ c$

S-606.00



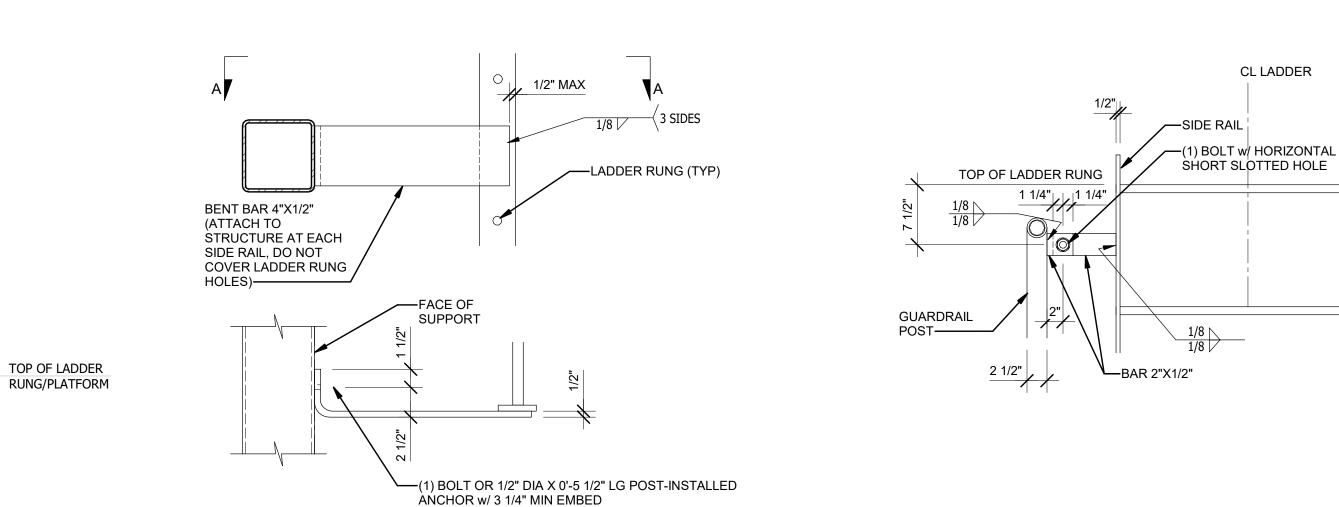


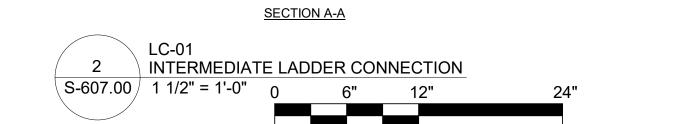




1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.

ISSUED FOR PERMIT

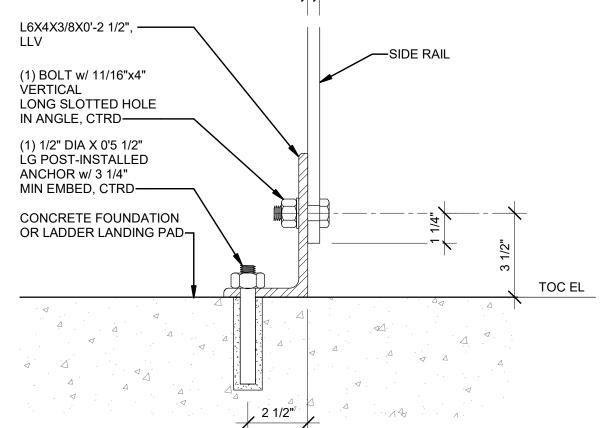


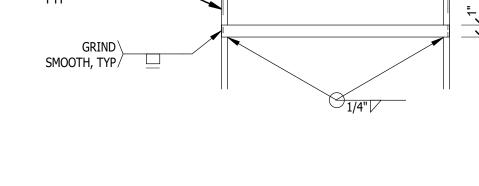


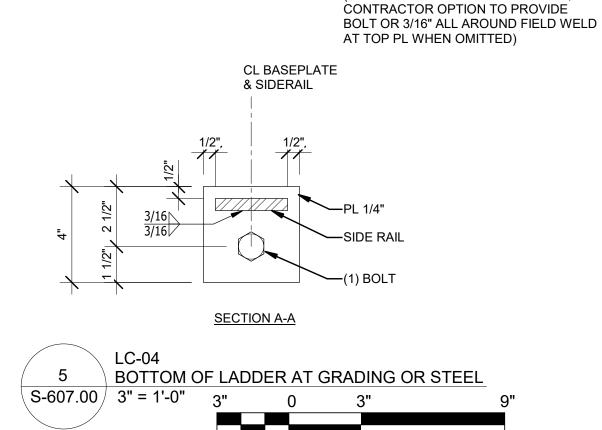
—SIDE RAIL

(OMIT WHEN LANDING ON STEEL PL,





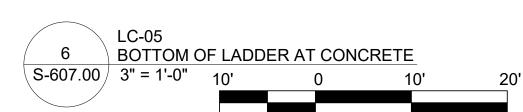


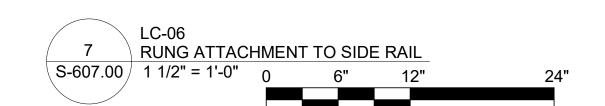


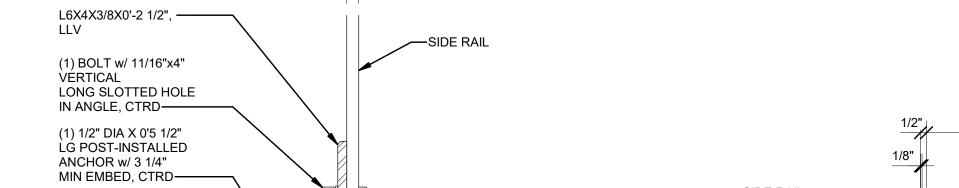
PL 1/4"——

TOP OF TOP HOOP

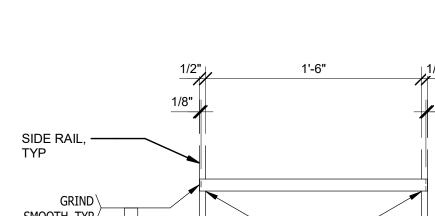
GUARDRAIL POST-













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A INTERIM SUBMISSION

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12/12/2022

DJF WA 09/13/2022 DRW BY CHK BY DATE

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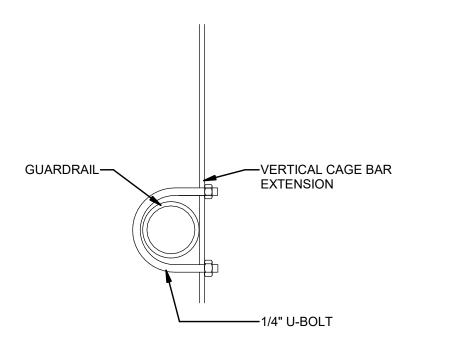
Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

LADDER TYPICAL DETAILS

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
	207.00
1 . 7 -r	7() / ()()

3-001.00 CADD FILE N0 Autodesk Docs://CHPE Astoria/CHA-KIE-000-XX-M2-S-001.rvt



4 TOP OF STEP THRU ACCESS LADDER CONNECTION

-SAFETY GATE

1" AT CAGE LOCATIONS

LADDER CAGE

TOP OF TOP HOOP

2'-0"

CL LADDER

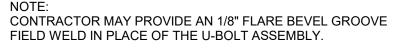
LD-10 TOP OF LADDER AT SAFETY GATE

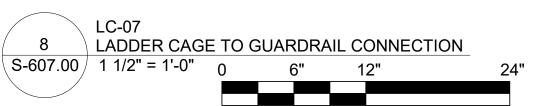
S-607.00 / 1" = 1'-0"

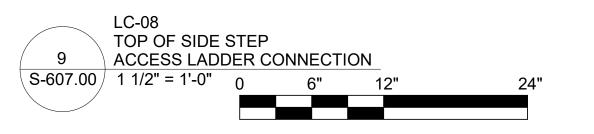
(1) BOLT w/ HORIZONTAL SHORT SLOTTED HOLE—

S-607.00 1" = 1'-0" 1' 6" 0

GUARDRAIL POST----

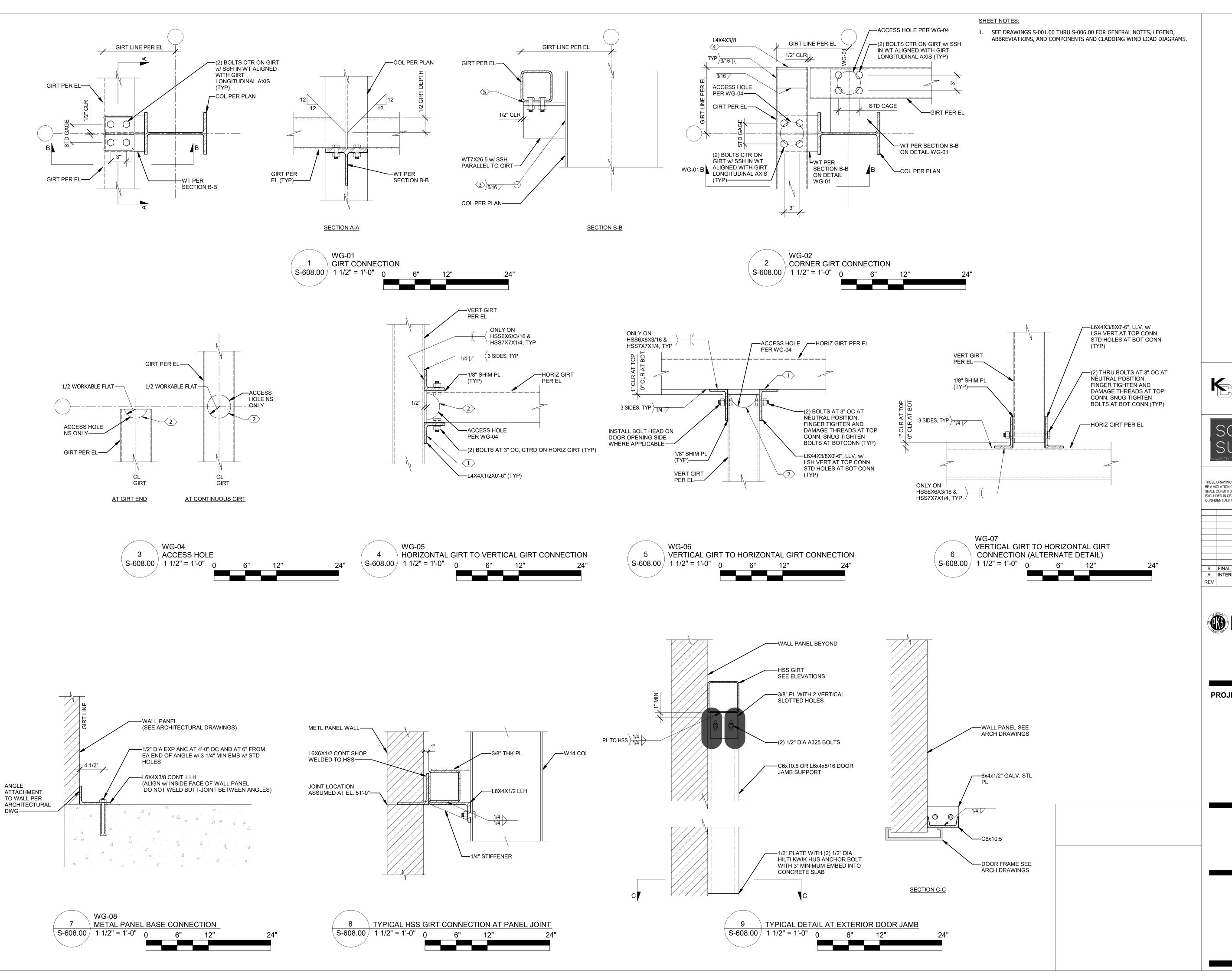






(1) BOLT w/ HORIZONTAL TOP HOOP OF SHORT SLOTTED HOLE LADDER CAGE

─VERTICAL CAGE BAR (TYP)



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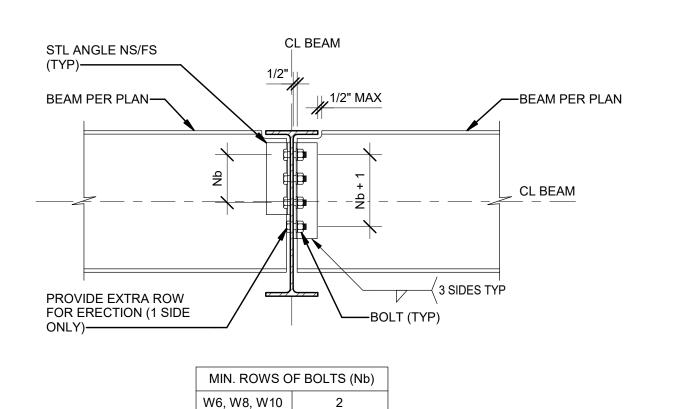


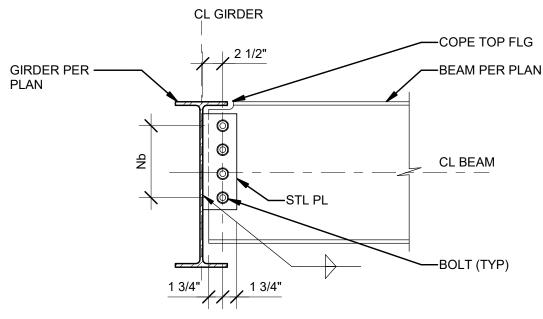
Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

TYPICAL GIRT DETAILS

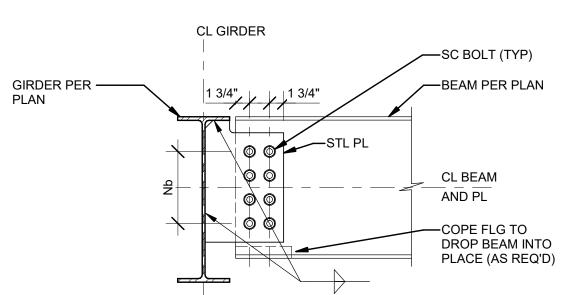
12/12/2022 PROJECT NO 105121 DRAWING BY D. FLYNN CHECKED BY W. ABBASSI DRAWING NO





BEAM TO GIRDER SHEAR PLATE

⟨S-609.00 / 1" = 1'-0"



SHEET NOTES:

1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND,

2. 3/4" DIAMETER A325N BOLTS WITH 1 1/2" MINIMUM EDGE DISTANCE, UNO.

3. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR

4. WITH THE EXCEPTION OF BOLTED ANGLES, THE WORKLINE FOR ALL

STANDARD HOLES @ 3" BOLT SPACING, UNO.

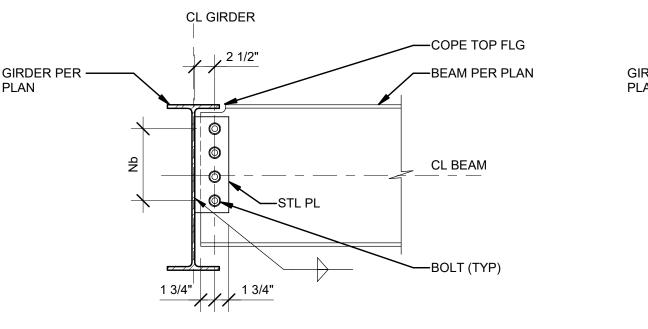
BRACING MEMBERS SHALL BE AT THE CENTROID.

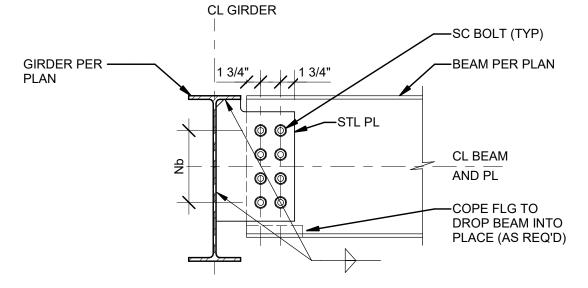
BY THE GENERAL NOTES.

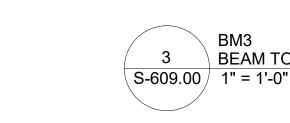
ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.

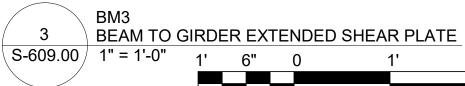
ERECTOR. BOLTS SHOWN AS HEX BOLTS MAY BE TC BOLTS AS DETERMINED

ISSUED FOR PERMIT



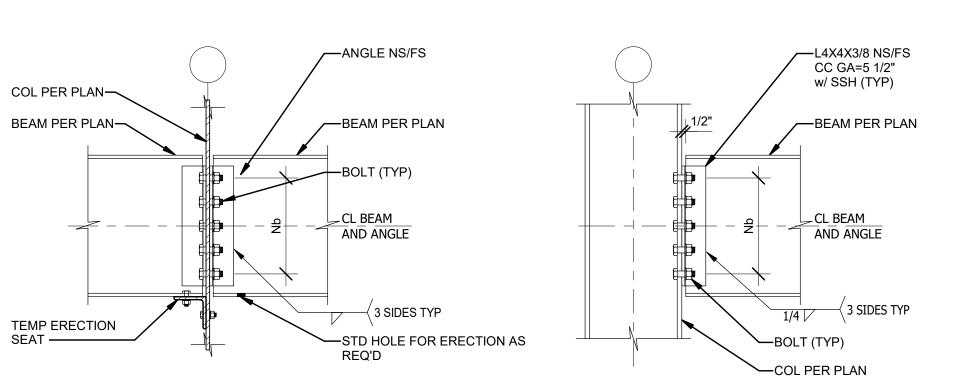


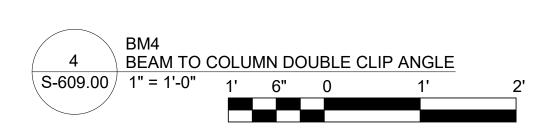


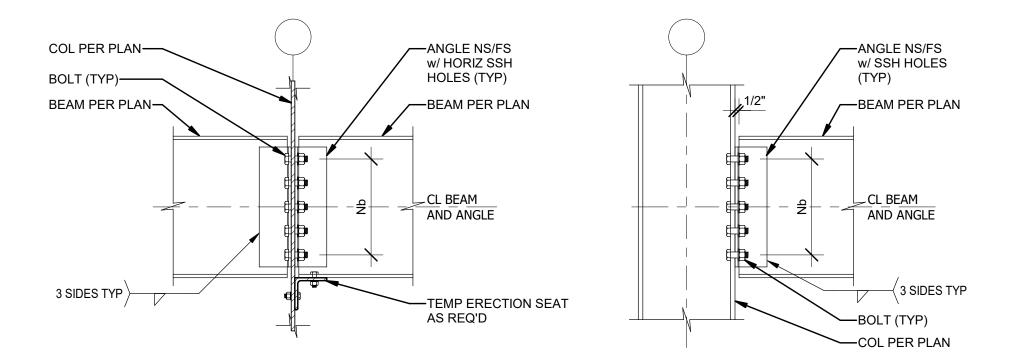


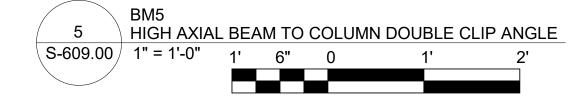
BEAM TO BEAM DBL CLIP ANGLE W/ TOP COPE ONLY S-609.00 | 1" = 1'-0"

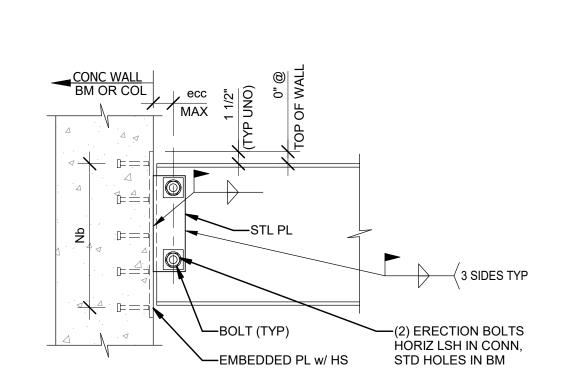
W12, W14 W16 W18, W21 W24, W27



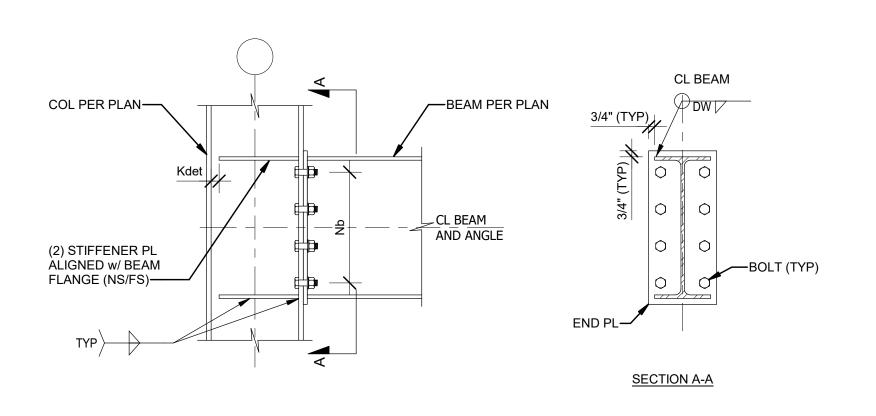


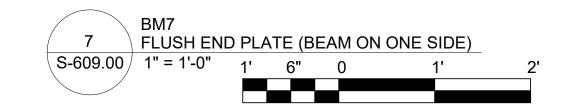














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В	FINAL SUBMISSION	DJF	WA	12/12/2022
Α	INTERIM SUBMISSION	DJF	WA	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



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Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

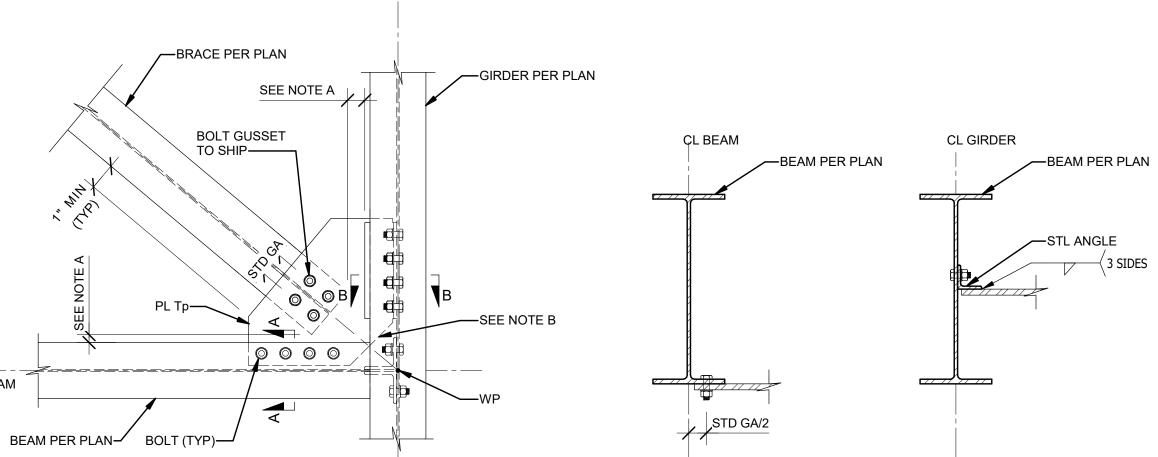
STEEL BEAM TYPICAL CONNECTIONS

> 12/12/2022 PROJECT NO 105121

> > CHECKED BY W. ABBASSI DRAWING NO

D. FLYNN

- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. 3/4" DIAMETER A325N BOLTS WITH 1 1/2" MINIMUM EDGE DISTANCE, UNO. STANDARD HOLES @ 3" BOLT SPACING, UNO.
- 3. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR ERECTOR. BOLTS SHOWN AS HEX BOLTS MAY BE TC BOLTS AS DETERMINED BY THE GENERAL NOTES.
- 4. WITH THE EXCEPTION OF BOLTED ANGLES, THE WORKLINE FOR ALL BRACING MEMBERS SHALL BE AT THE CENTROID.



CL GIRDER

NOTE:

A. DIMENSION FROM BRACE TO EITHER
BEAM OR EDGE OF PLATE SHALL BE SET

B. MINIMUM 30 DEGREE ANGLE BETWEEN

BRACE AND BEAMS

BOLT (TYP)—

NOTE A

BRACE PER

TO SHIP-

BEAM PER

PLAN----

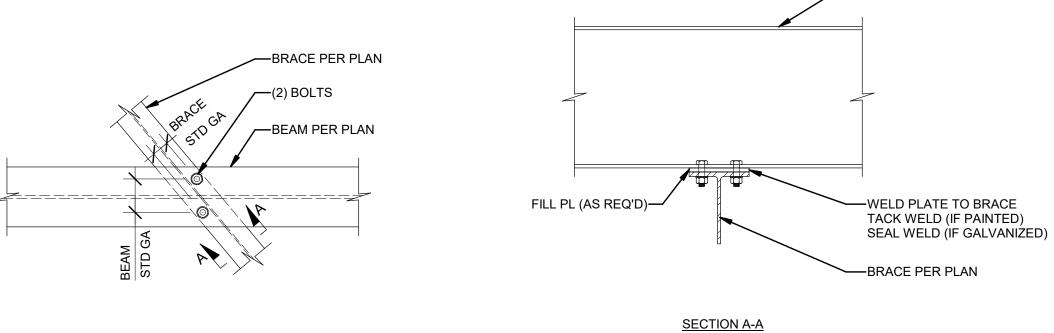
SECTION B-B

—BEAM PER PLAN

-STL ANGLE

WT HORIZONTAL BRACE AT COLUMN BEAM PER PLAN

—BEAM PER PLAN



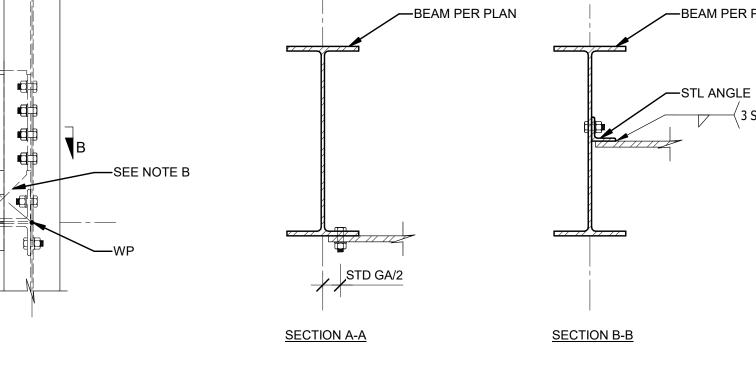
HORIZONTAL BRACE TO BOTTOM OF BEAM

S-610.00 / 1" = 1'-0" 1' 6" 0

-BEAM PER PLAN

SECTION A-A



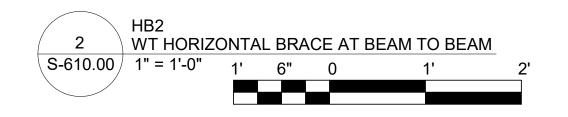


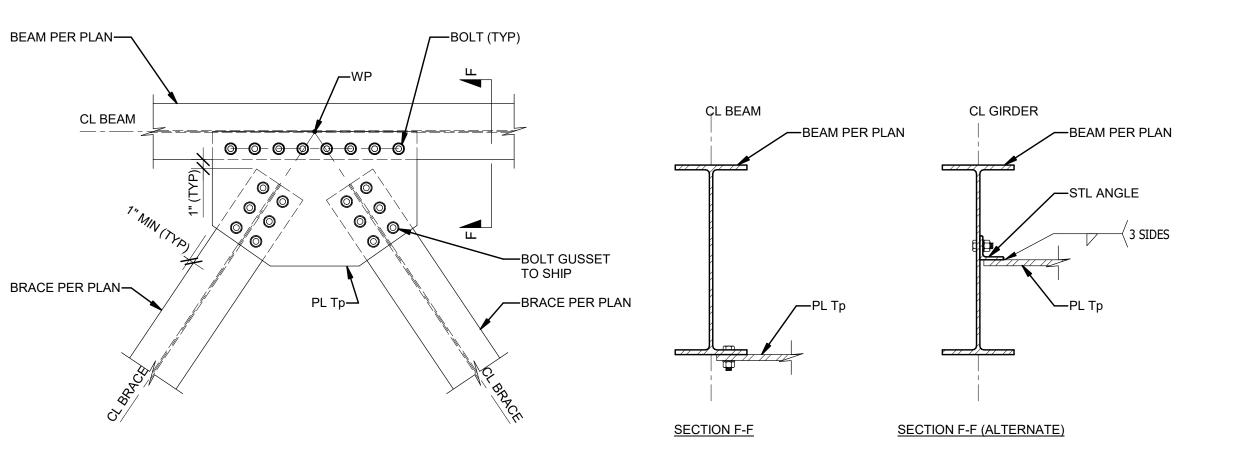
NOTES:
A. DIMENSION FROM BRACE TO EITHER BEAM OR ANGLE SHALL BE SET AT 1". B. FOR SHEAR TAB, EXTENDED SHEAR TAB AMD SINGLE ANGLE ADJACENT CONNECTIONS COPE GUSSET PLATE AS REQUIRED.

BEAM

BEAM PER PLAN

SECTION B-B (SLOPED)







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ISSUED FOR PERMIT

В	FINAL SUBMISSION	DJF	WA	12/12/2022
Α	INTERIM SUBMISSION	DJF	WA	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



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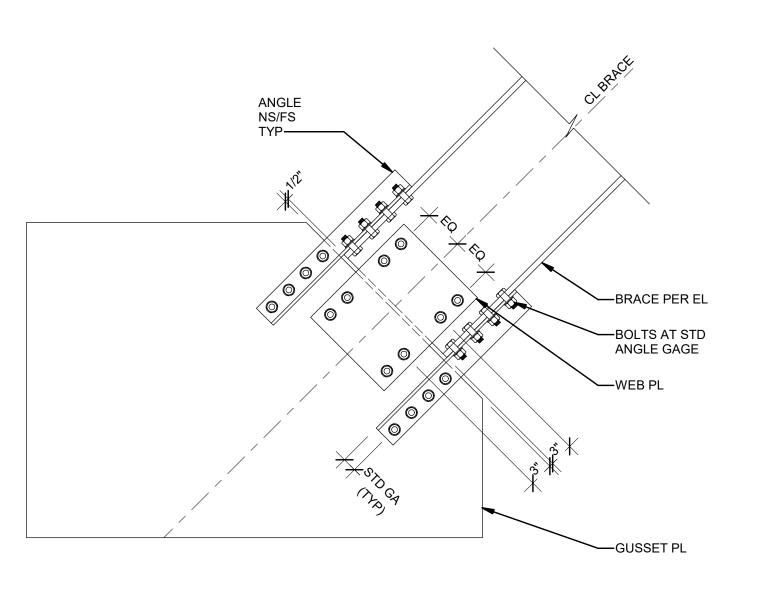


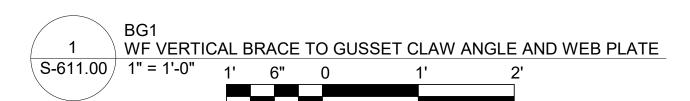
Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

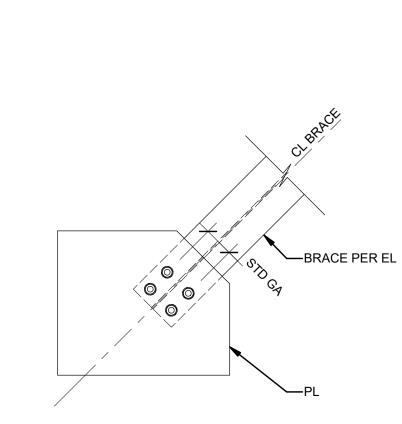
> STEEL HB TYPICAL **DETAILS**

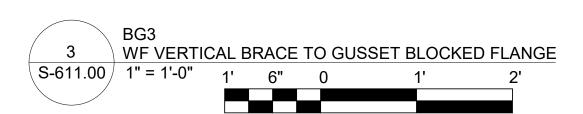
DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	
	W. ABBASSI
DRAWING NO	4000
S-6	1()()()



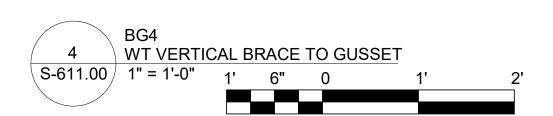


SECTION A-A





BRACE PER EL



SHEET NOTES:

─BRACE PER EL

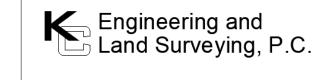
FILL AS REQ'D

GUSSET PL

BG2 WF VERTICAL BRACE TO GUSSET CHANNEL

- SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. 3/4" DIAMETER A325N BOLTS WITH 1 1/2" MINIMUM EDGE DISTANCE, UNO. STANDARD HOLES @ 3" BOLT SPACING, UNO.
- 3. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR ERECTOR. BOLTS SHOWN AS HEX BOLTS MAY BE TC BOLTS AS DETERMINED BY THE GENERAL NOTES.
- 4. WITH THE EXCEPTION OF BOLTED ANGLES, THE WORKLINE FOR ALL BRACING MEMBERS SHALL BE AT THE CENTROID.





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Α	INTERIM SUBMISSION	DJF	WA	09/13/2022
REV	DESCRIPTION	DRW BY	CHK BY	DATE



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Astoria HVDC Converter Station

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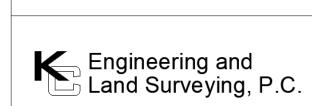
> STEEL BG TYPICAL CONNECTIONS

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
\mathbf{C}	11 00

S-611.00

- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
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REV	DESCRIPTION	DRW BY	CHK BY	DATE



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Astoria HVDC Converter Station

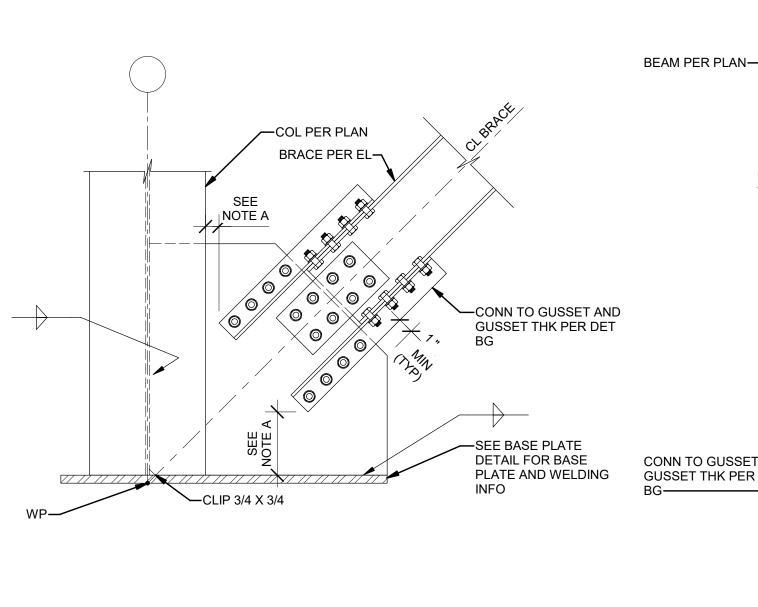
31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

TYPICAL VERTICAL BRACE **CONNECTIONS**

1	
DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	

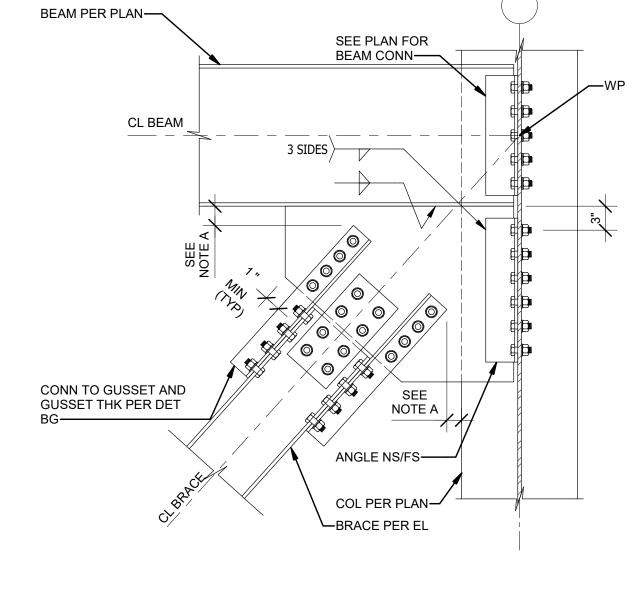
S-612.00

CADD FILE NO
Autodesk Docs://CHPE
Astoria/CHA-KIE-000-XX-M2-S-001.rvt

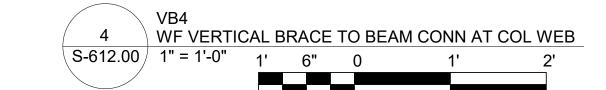


BEAM CONN-CL BEAM 3 SIDES CONN TO GUSSET AND GUSSET THK PER DET NOTE A ANGLE NS/FS--BRACE PER EL COL PER PLAN-

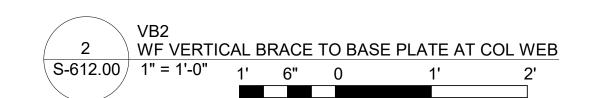
SEE PLAN FOR

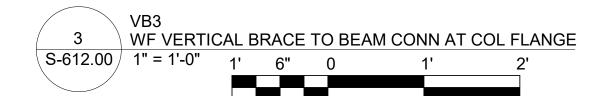


A. NOTEDIMENSION FROM ANGLE TO EITHER COLUMN OR BEAM SHALL BE SET AT 1".

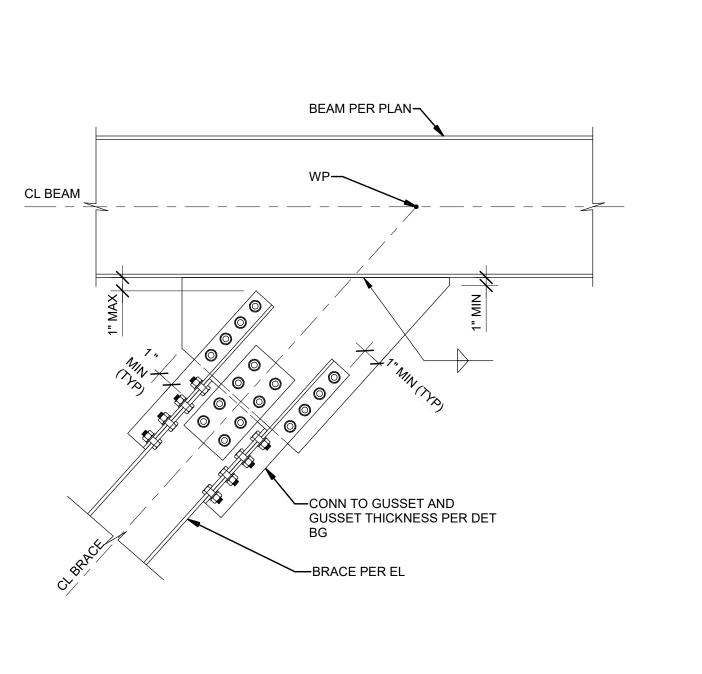


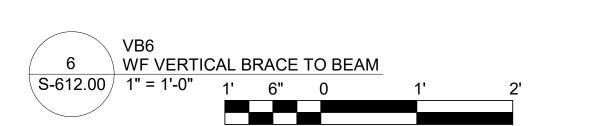
A. DIMENSION FROM ANGLE TO EITHER COLUMN OR BASE PLATE SHALL BE SET AT 1".

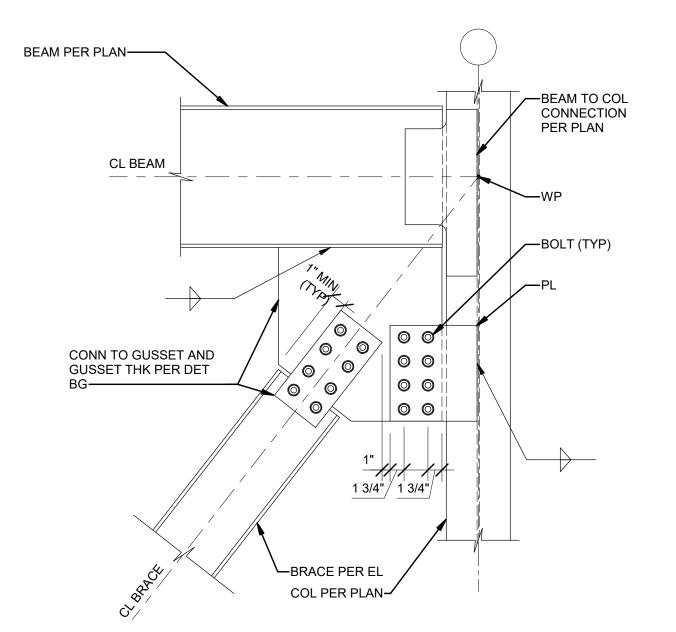


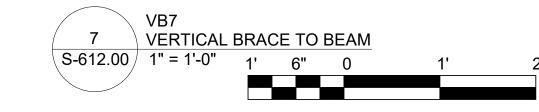


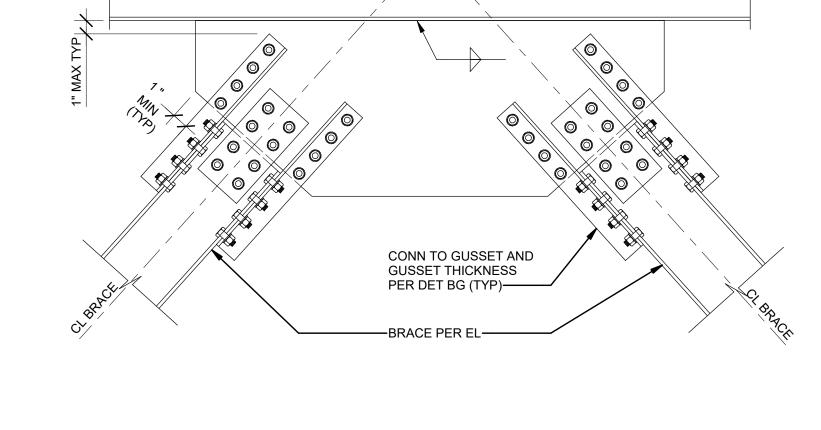
A. DIMENSION FROM ANGLE TO EITHER GUSSET CONN ANGLE OR BEAM SHALL BE SET AT 1".











BEAM PER PLAN—

COL PER PLAN-

NOTE A

A. DIMENSION FROM ANGLE TO EITHER COLUMN OR BASE PLATE

WF VERTICAL BRACE TO BASE PLATE AT COL FLANGE

SHALL BE SET AT 1".

CLIP 3/4 X 3/4

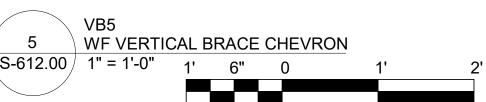
-BRACE PER EL

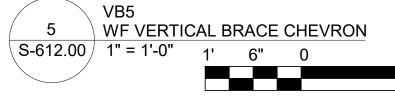
CONN TO GUSSET AND GUSSET THK PER DET

SEE BASE PLATE DETAIL FOR BASE

CL BEAM

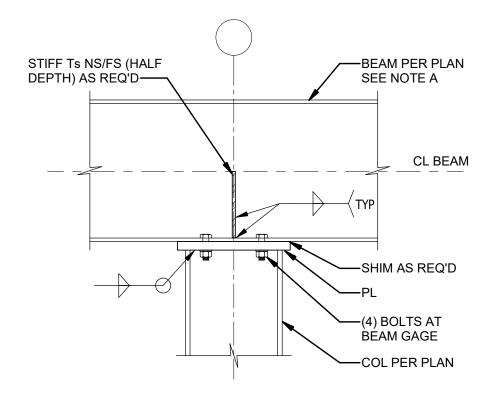
PLATE AND WELDING





- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. 3/4" DIAMETER A325N BOLTS WITH 1 1/2" MINIMUM EDGE DISTANCE, UNO. STANDARD HOLES @ 3" BOLT SPACING, UNO.
- 3. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR ERECTOR. BOLTS SHOWN AS HEX BOLTS MAY BE TC BOLTS AS DETERMINED BY THE GENERAL NOTES.
- 4. WITH THE EXCEPTION OF BOLTED ANGLES, THE WORKLINE FOR ALL BRACING MEMBERS SHALL BE AT THE CENTROID.





NOTE:

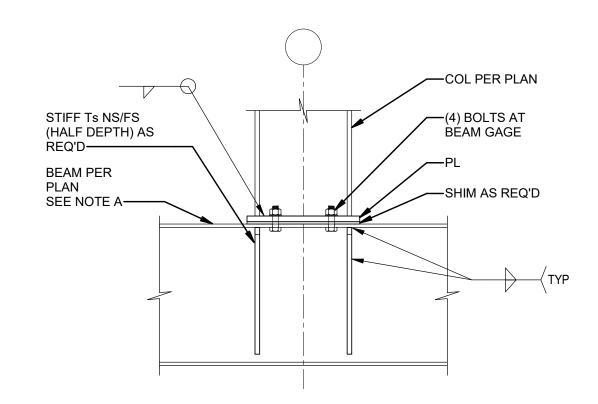
A. CONNECTION OF MEMBER FRAMING INTO BEAM NOT SHOWN FOR

W14X145 - W14x176

W14X211

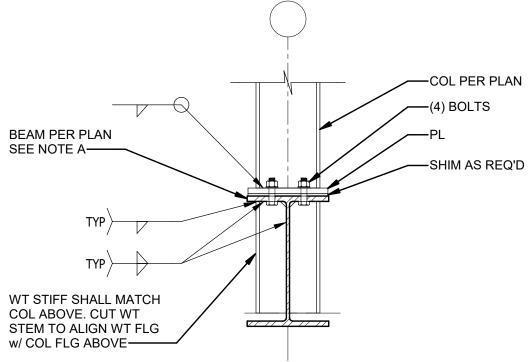
380

680



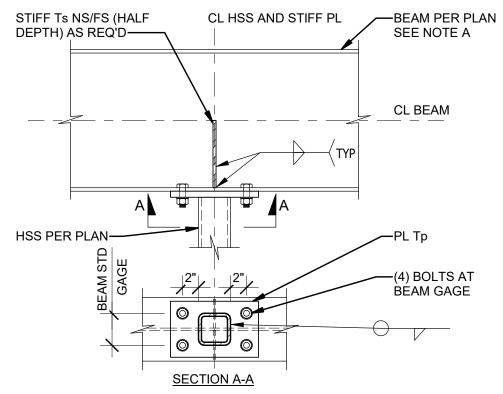
NOTE:
A. CONNECTION OF MEMBER FRAMING INTO BEAM NOT SHOWN FOR

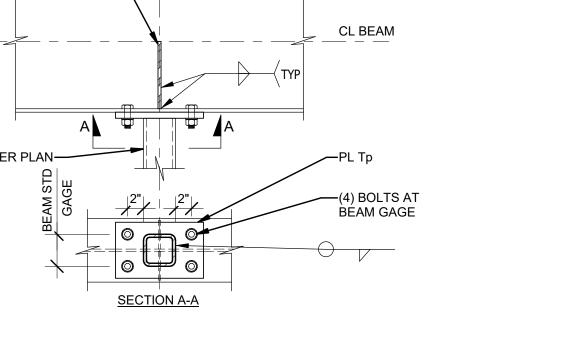
FINISH TO BEAR

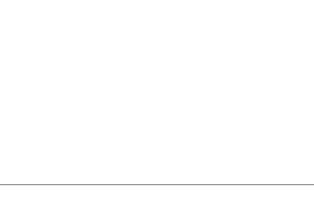


NOTE:
A. CONNECTION OF MEMBER FRAMING INTO BEAM NOT SHOWN FOR

\S-613.00 \/ 1" = 1'-0" 1'



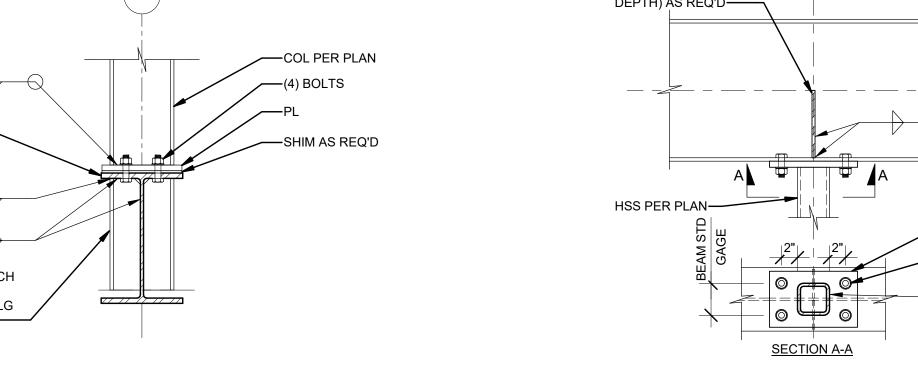


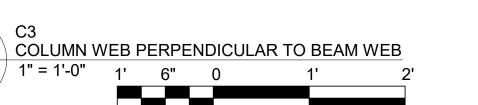


NOTE:

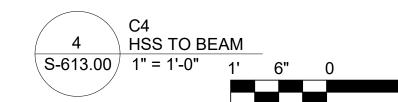
A. CONNECTION OF MEMBER FRAMING INTO BEAM NOT SHOWN FOR











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PROJECT



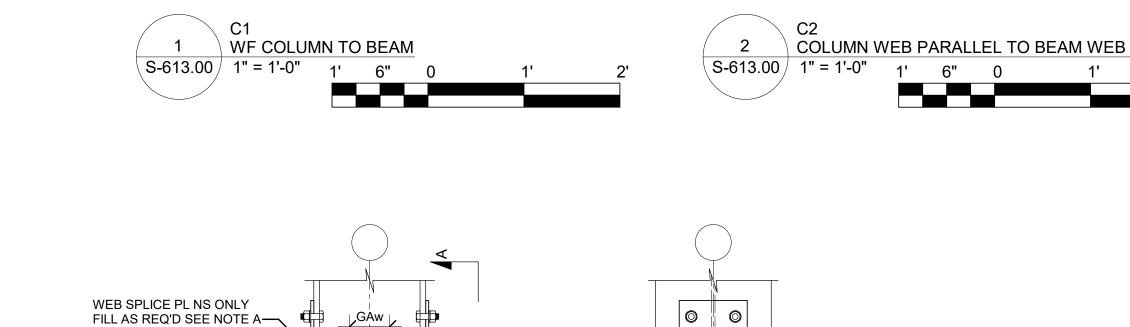
Astoria HVDC Converter Station

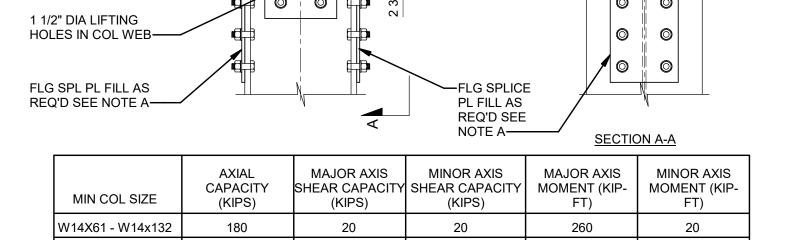
31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

STEEL COLUMN TYPICAL **CONNECTIONS**

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
	1000

S-613.00 CADD FILE NO
Autodesk Docs://CHPE
Astoria/CHA-KIE-000-XX-M2-S-001.rvt





100

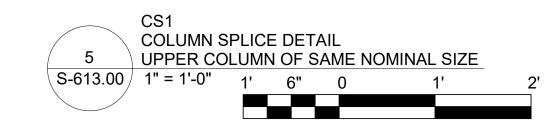
NOTE:
A. ALL BOLTS IN THIS CONNECTION SHALL BE FULLY PRETENSIONED

420

100

20

100



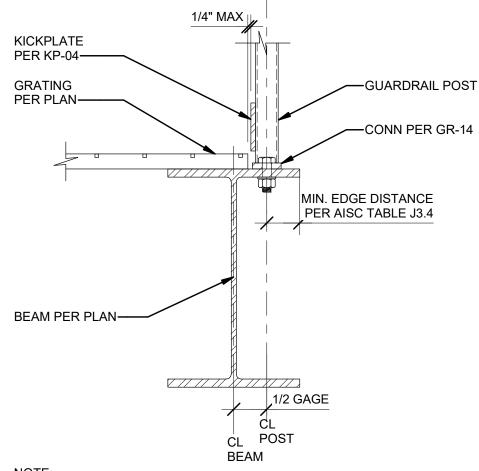
10

KICKPLATE

PER KP-04-

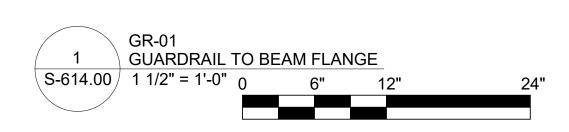
- SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. 7/8" DIAMETER A325N BOLTS WITH 1 1/2" MINIMUM EDGE DISTANCE WITH STANDARD HOLES @ 3" BOLT SPACING, UNO.
- 3. CONNECTIONS ARE SCHEMATICALLY SHOWN FOR DESIGN INTENT. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR ERECTOR. BOLTS SHOWN AS HEX BOLTS MAY BE TC BOLTS AS DETERMINED BY THE GENERAL NOTES.

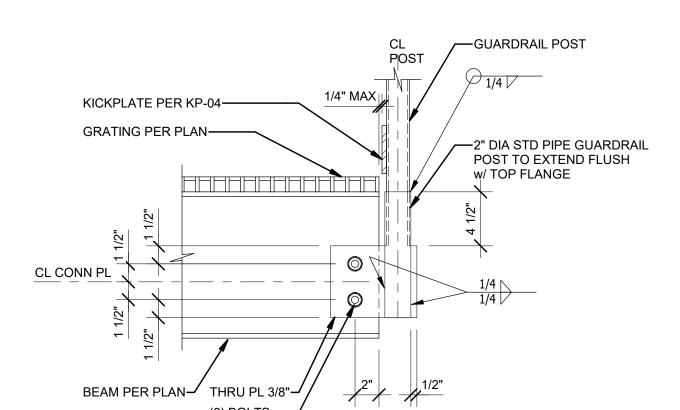


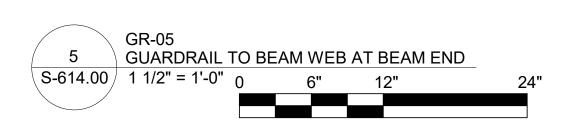


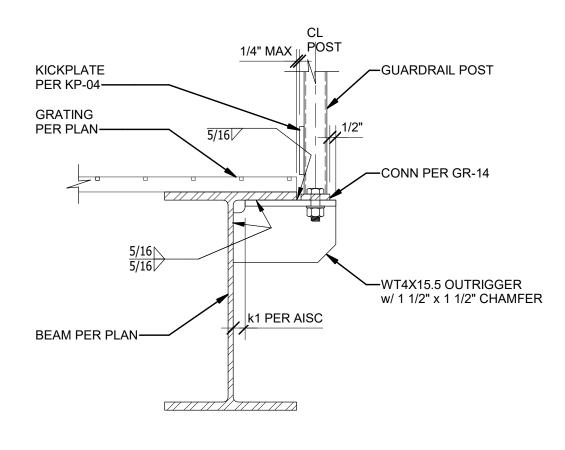
NOTE:

A. WHEN MULTIPLE BEAM GAGES EXIST ON A CONSECUTIVE RUN OF BEAMS, THE SMALLEST GAGE SHALL GOVERN THE LOCATION OF THE GUARDRAIL.



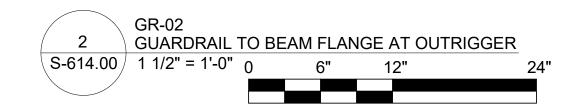


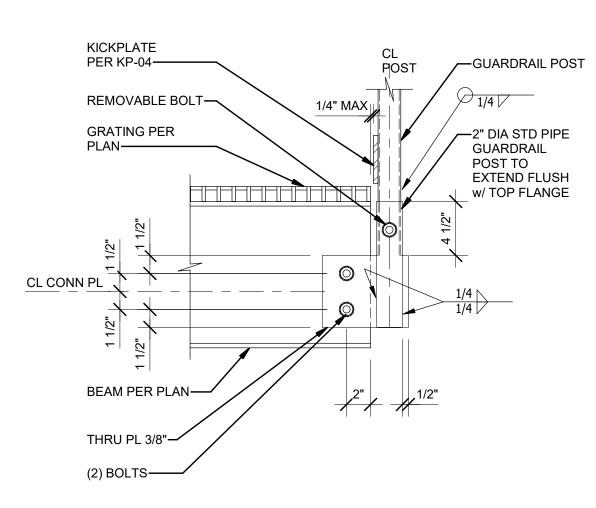




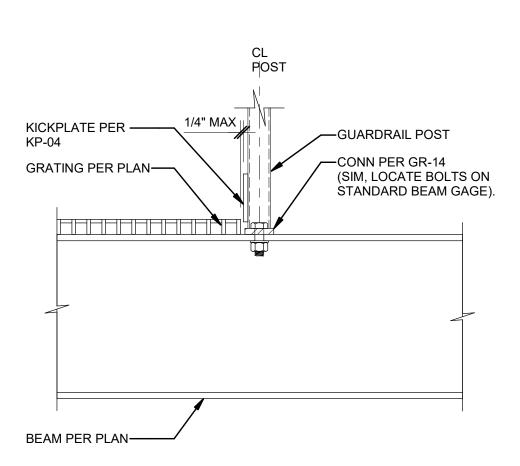
NOTE:

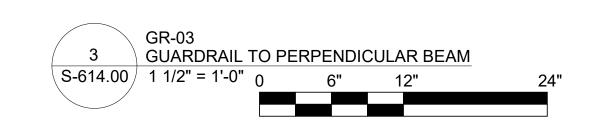
A. WHEN MULIPLE BEAM FLANGE WIDTHS EXIST ON A CONSECUTIVE RUN OF BEAMS, THE LARGEST FLANGE SHALL GOVERN THE LOCATION OF THE GUARDRAIL.

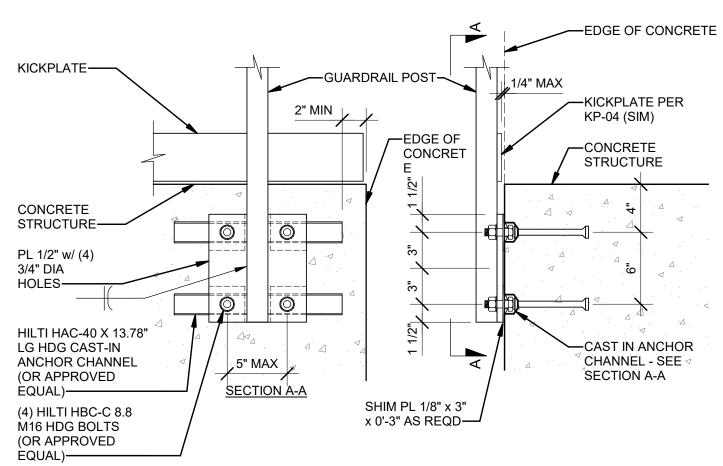


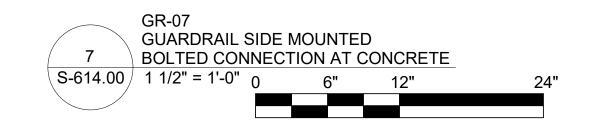


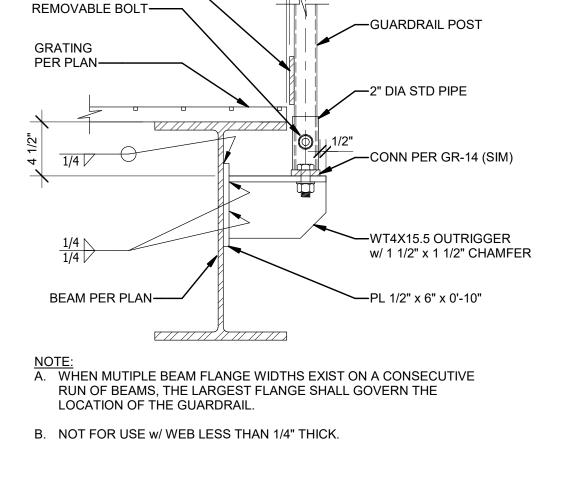


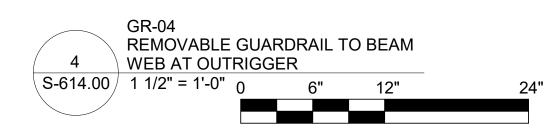


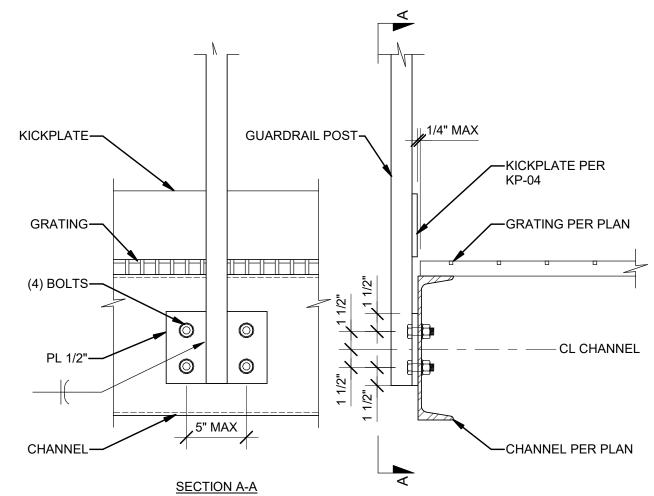


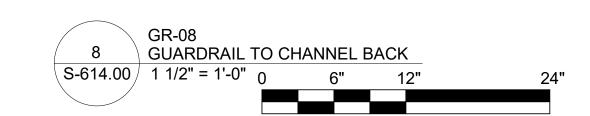


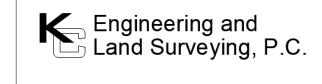




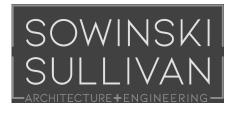








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REV	DESCRIPTION	DRW BY	CHK BY	DATE
	•	•	•	•



Hitachi Energy901 Main Campus Drive Raleigh, North Carolina 27606

PROJECT



Astoria HVDC Converter Station

31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

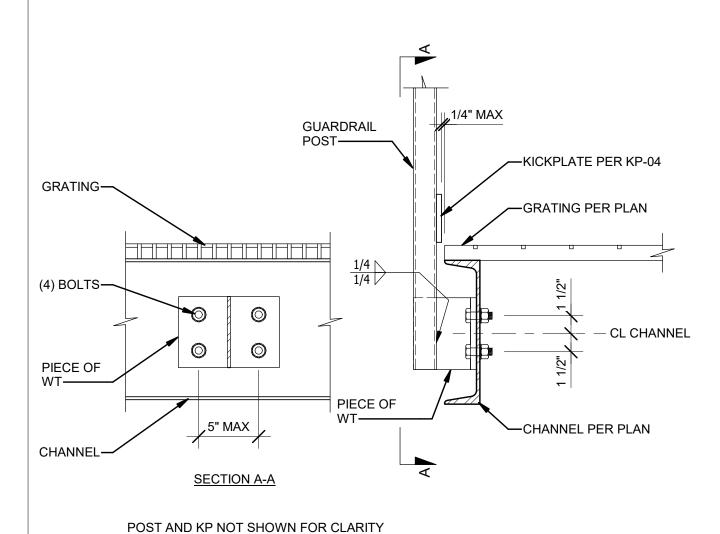
GUARDRAIL TYPICAL CONNECTIONS

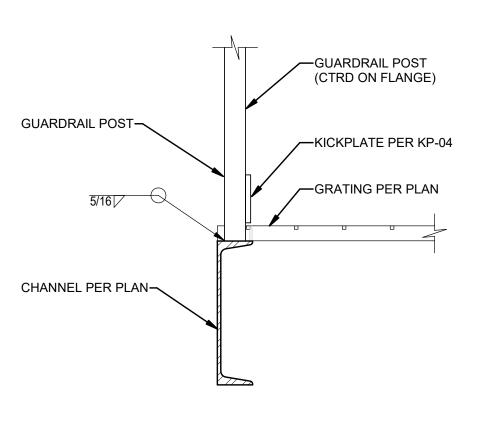
DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	

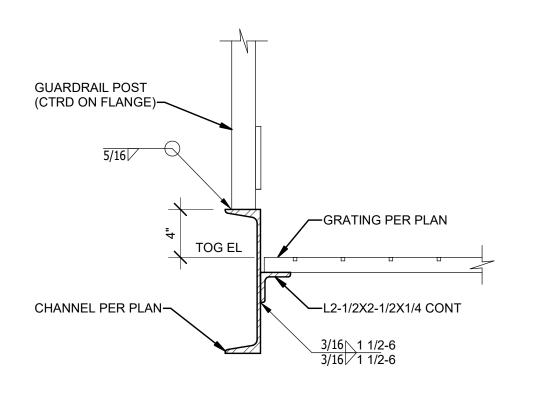
S-614.00

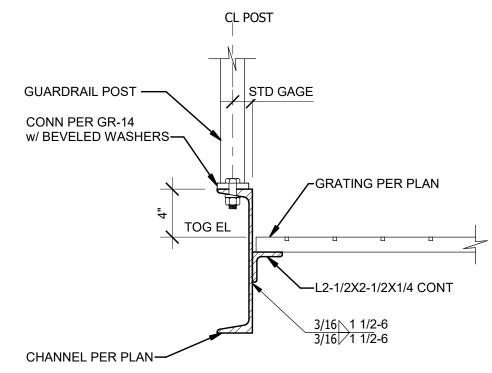
- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. 7/8" DIAMETER A325N BOLTS WITH 1 1/2" MINIMUM EDGE DISTANCE WITH STANDARD HOLES @ 3" BOLT SPACING, UNO.
- 3. CONNECTIONS ARE SCHEMATICALLY SHOWN FOR DESIGN INTENT. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR ERECTOR. BOLTS SHOWN AS HEX BOLTS MAY BE TC BOLTS AS DETERMINED BY THE GENERAL

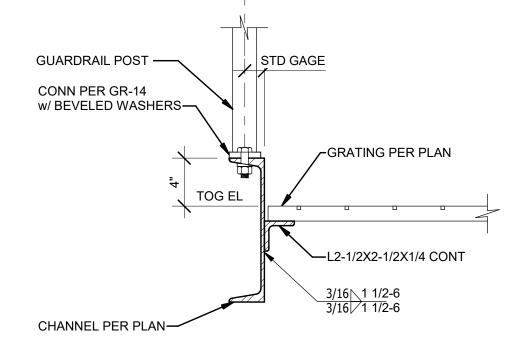


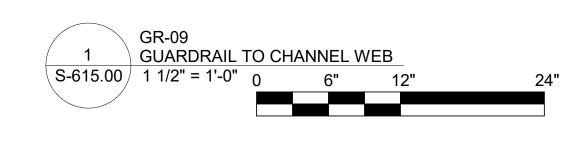


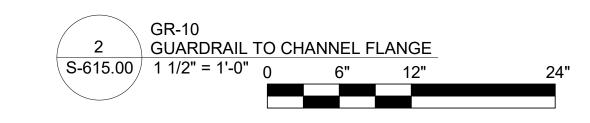




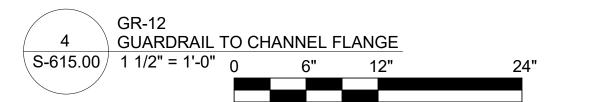


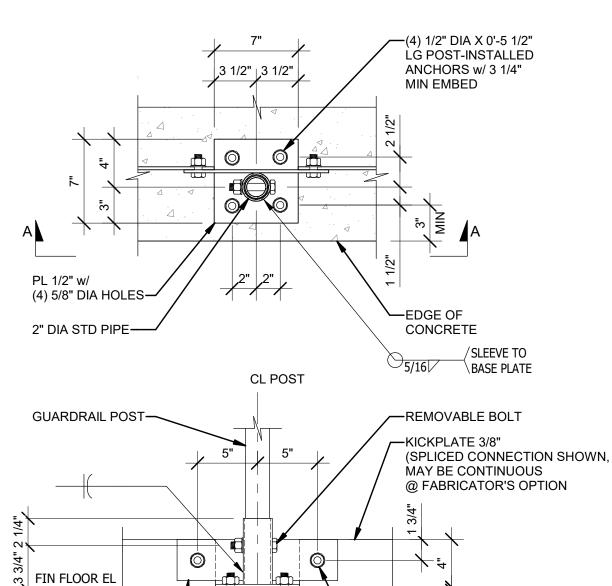


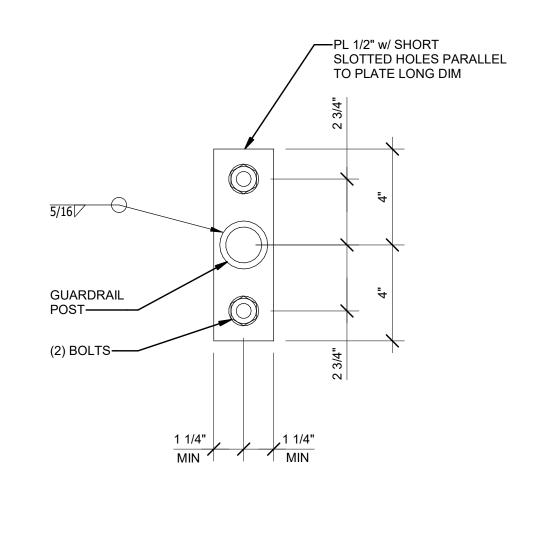


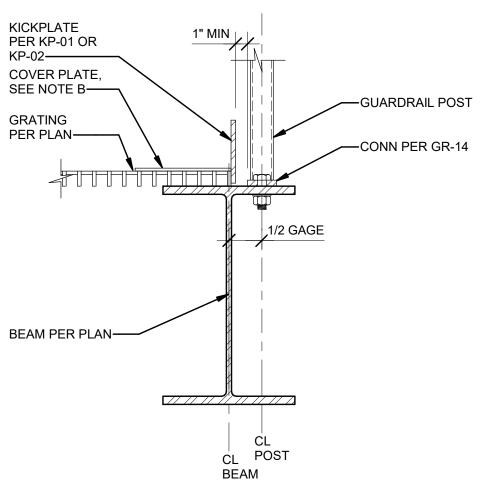


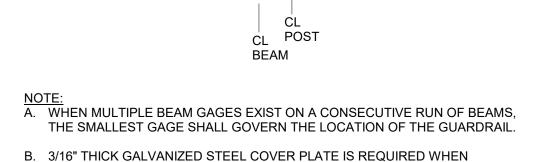








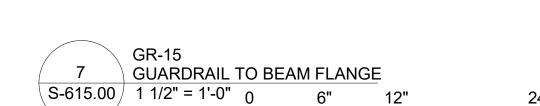


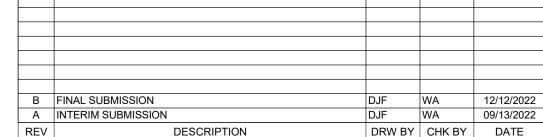


GRATING SPANS PARALLEL TO BEAM. COVER PLATES SHOULD EXTEND

(1) BEARING BAR PAST EDGE OF BEAM FLANGE. SEAL WELD PLATE TO

GRATING BARS, ALL SIDES.





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EXCLUDED IN OBTAINING THIS INFORMATION FROM A THIRD PARTY. IF THE RECIPIENT IS NOT IN AGREEMENT WITH THE OBLIGATION OF

BE A VIOLATION OF THIS CONFIDENTIALITY REQUIREMENT AND SUBJECT THE VIOLATOR TO LIABILITY, REVIEW OF THESE MATERIALS BY RECEIPT SHALL CONSTITUTE ACCEPTANCE OF THESE TERMS AND THE TERMS OF ANY UNDERLYING CONFIDENTIALITY AGREEMENT WE MAY HAVE



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12/12/2022

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25 Mohawk Avenue

Sparta, NJ 07871

SUITE 1604 New York, NY 10001



Astoria HVDC Converter Station

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> **GUARDRAIL TYPICAL** CONNECTIONS

DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
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CADD FILE NO 45 of 49 Autodesk Docs://CHPE
Astoria/CHA-KIE-000-XX-M2-S-001.rvt

	GR-13					
	GUARDRAIL 7	ГОР М	DUNTED			
5	BOLTED CON	INECTI	ON AT B	ASE		
S-615.00	1 1/2" = 1'-0"	0	6"	 12"	24"	
				- 		

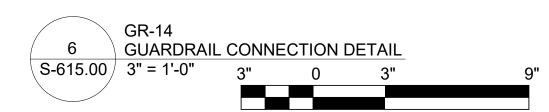
(2) BOLTS

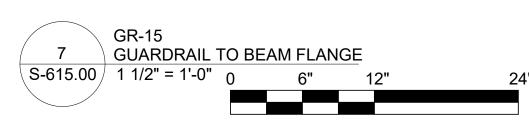
SECTION A-A

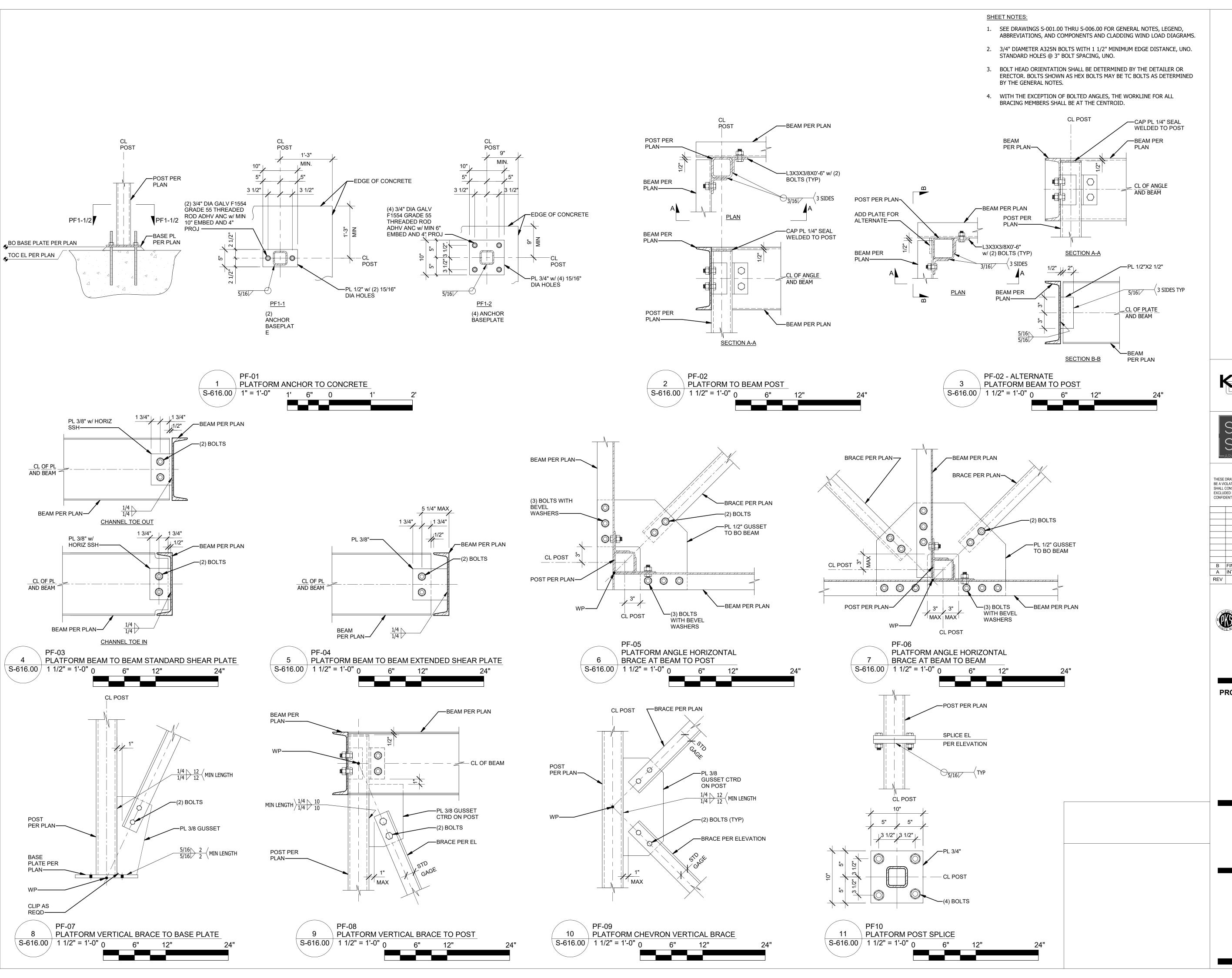
PL 3/8" w/ HORIZONTAL

SHORT SLOTTED

HOLES-







ISSUED FOR PERMIT

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В	FINAL SUBMISSION	DJF	WA	12/12/2022
Α	INTERIM SUBMISSION	DJF	WA	09/13/2022
RE\	DESCRIPTION	DRW BY	CHK BY	DATE



Hitachi Energy901 Main Campus DriveRaleigh, North Carolina 27606

PROJECT



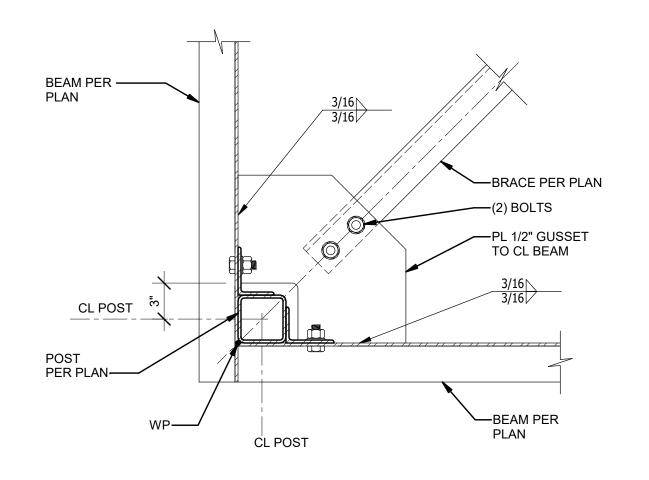
Astoria HVDC Converter Station

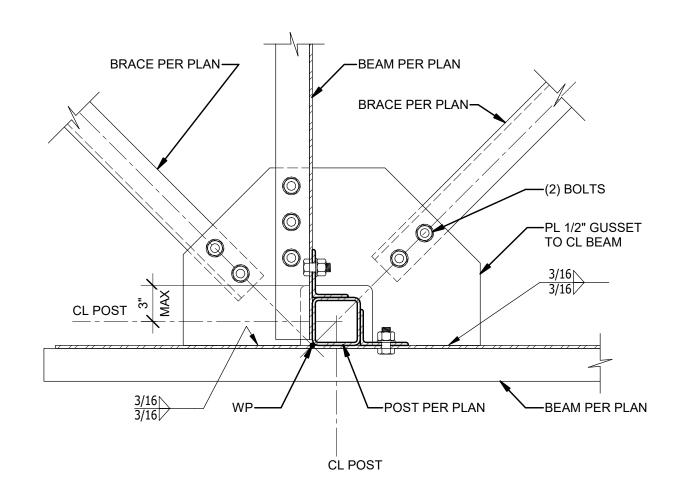
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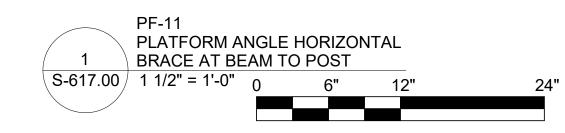
STEEL PLATFORM
TYPICAL CONNECTIONS

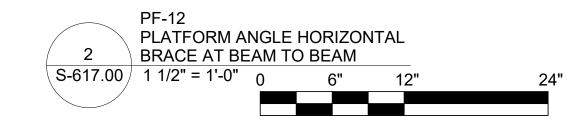
DATE	12/12/2022
PROJECT NO	105121
DRAWING BY	D. FLYNN
CHECKED BY	W. ABBASSI
DRAWING NO	
S-61	6.00

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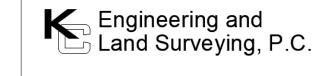






- 1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.
- 2. 3/4" DIAMETER A325N BOLTS WITH 1 1/2" MINIMUM EDGE DISTANCE, UNO. STANDARD HOLES @ 3" BOLT SPACING, UNO.
- 3. BOLT HEAD ORIENTATION SHALL BE DETERMINED BY THE DETAILER OR ERECTOR. BOLTS SHOWN AS HEX BOLTS MAY BE TC BOLTS AS DETERMINED BY THE GENERAL NOTES.
- 4. WITH THE EXCEPTION OF BOLTED ANGLES, THE WORKLINE FOR ALL BRACING MEMBERS SHALL BE AT THE CENTROID.





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PROJECT



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STEEL PLATFORM
TYPICAL CONNECTIONS

 DATE
 12/12/2022

 PROJECT NO
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 DRAWING BY
 D. FLYNN

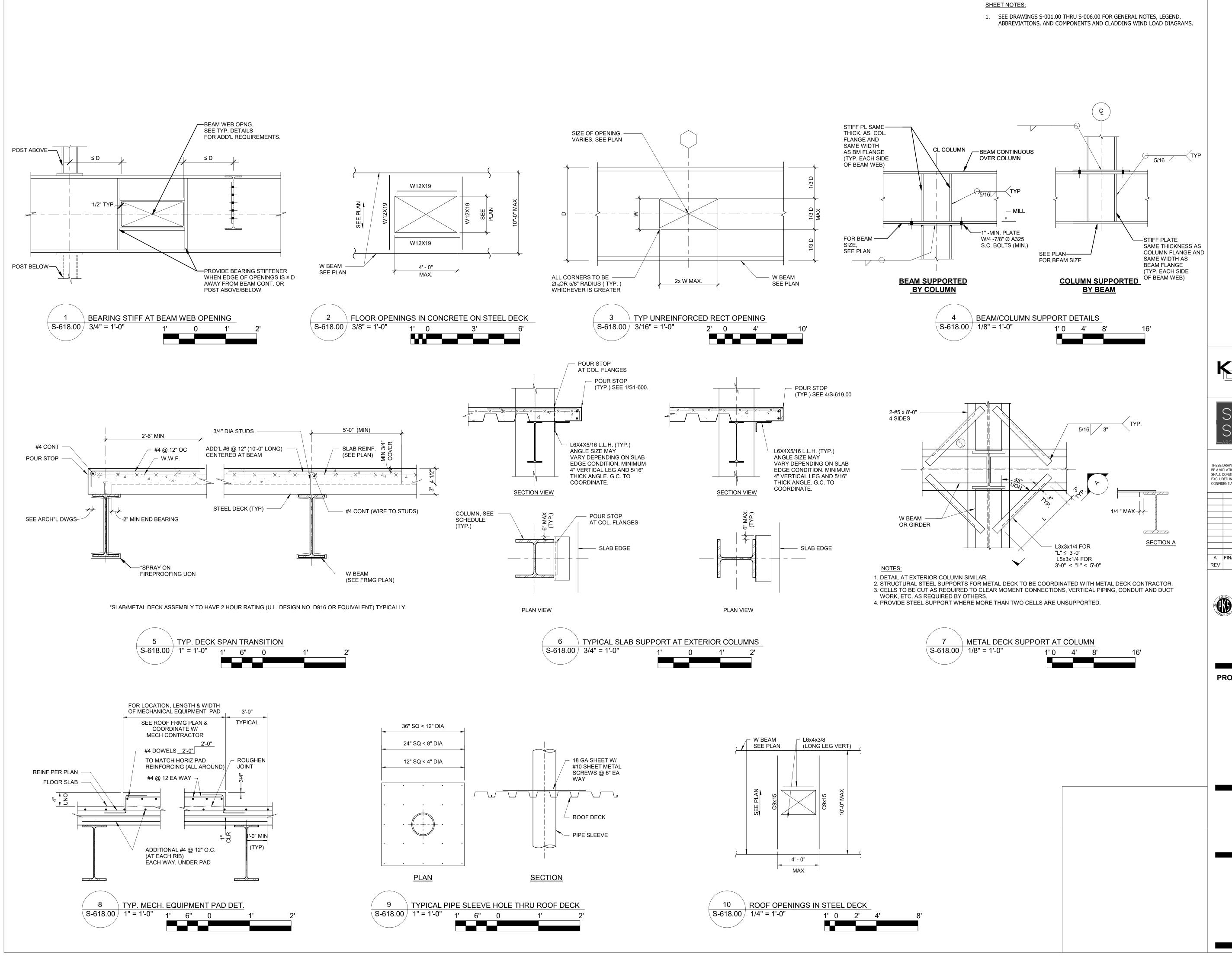
 CHECKED BY
 W. ABBASSI

 DRAWING NO

S-617.00

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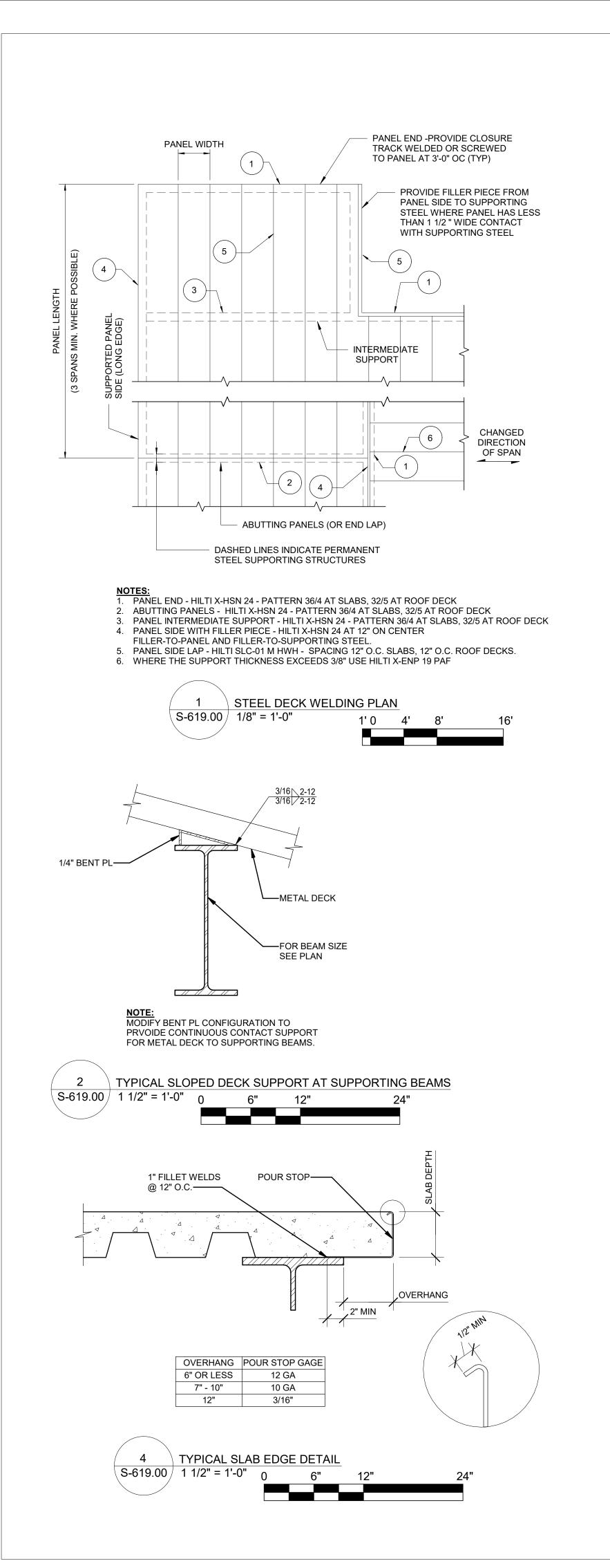


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TYPICAL METAL DECK **ATTACHMENT DETAILS**

> 12/12/2022 PROJECT NO 105121 DRAWING BY D. FLYNN CHECKED BY W. ABBASSI DRAWING NO



WELDED STUDS

AT CL OF BEAM

COMPOSITE DECK

SEE NOTES

TYP SECTION

DECK PERPENDICULAR TO BEAM

1. SEE DRAWINGS S-001.00 THRU S-006.00 FOR GENERAL NOTES, LEGEND, ABBREVIATIONS, AND COMPONENTS AND CLADDING WIND LOAD DIAGRAMS.

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REV DESCRIPTION

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Astoria HVDC Converter Station

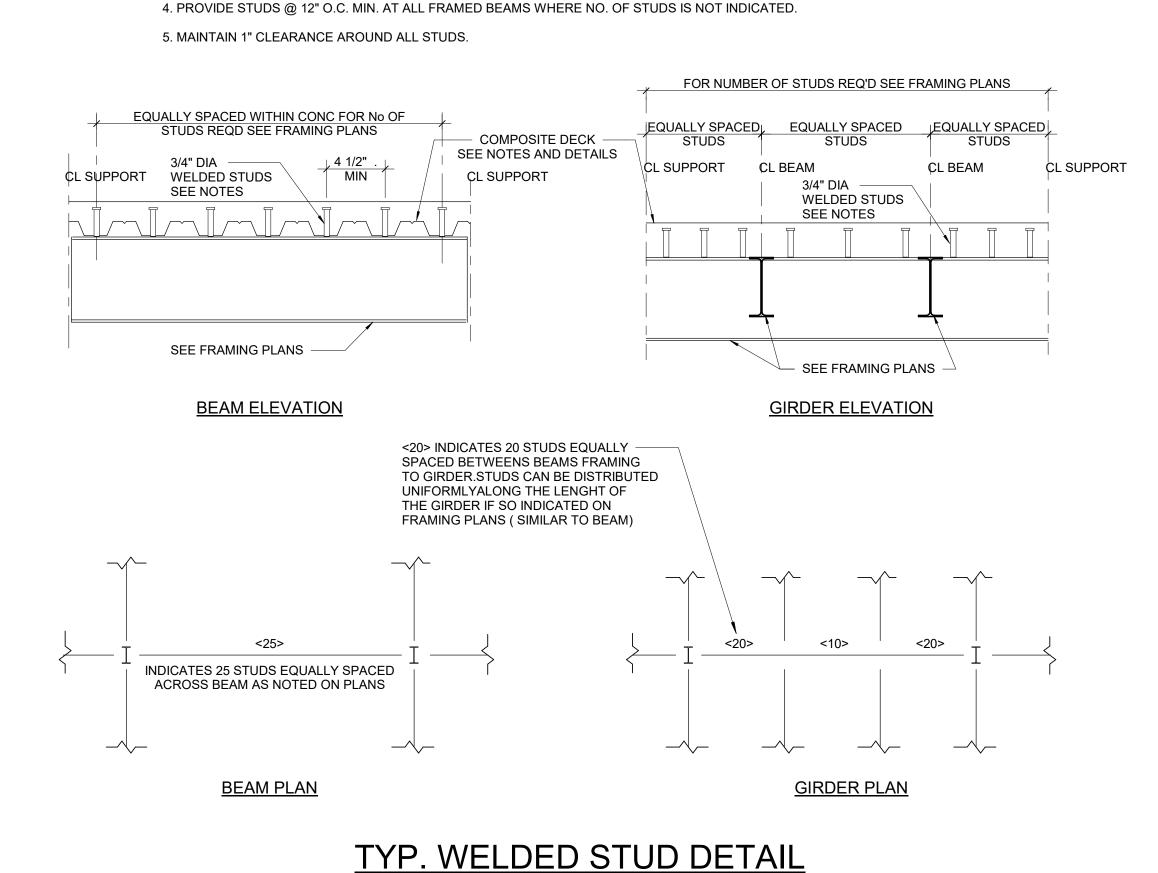
31-45 20th Avenue, Astoria, Queens NY 11105 Block #850 - Lot #310 - BIN #4624437

TYPICAL METAL DECK ATTACHMENT DETAILS

DATE 12/12/2022
PROJECT NO 105121
DRAWING BY D. FLYNN
CHECKED BY W. ABBASSI
DRAWING NO

S-619.00

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TYP. WELDED STUD DETAIL

S-619.00 / 3/4" = 1'-0"

NOTES:
1. ALL WELDED STUDS SHALL BE 3/4" DIA. HEADED STUDS. STUDS SHALL EXTEND 1 1/2" MINIMUM ABOVE TOP OF STEEL DECK.

2. WHERE THE NUMBER OF STUDS EXCEEDS THE NUMBER OF DECK FLUTES, PROVIDE MIN. ONE STUD PER FLUTE & DOUBLE STUDS AT EACH FLUTE STARTING AT EACH SUPPORT WORKING TOWARD CENTER OF THE BEAM UNTIL THE REQ'D NUMBER OF STUDS ARE

2 ROWS WHERE REQD (5 1/2" MIN BM WIDTH REQD

COMPOSITE DECK

1" MIN. TO 1 1/2" MAX

SEE NOTE 1 BELOW

PROVIDED. PROVIDE AN EQUAL NUMBER OF STUDS AT EACH SIDE OF BEAM SPAN CENTER LINE.

3. STUDS WELDED THROUGH STEEL DECKING MAY BE SUBSTITUTED FOR A PUDDLE WELD, TYPICAL

3/4" DIA WELDED STUDS AT CL OF

1 1/4" MIN 🛨

TYP SECTION

DECK PARALLEL TO BEAM

SEE PLAN